A report prepared for

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GEOTECHNICAL EXPLORATION
Liberty Development Project
Foggy Island Bay, Alaska

by

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SUMMARY

During March 1997, Duane Miller & Associates (DM&A) drilled and sampled 30 geotechnical borings in Foggy Island Bay for the proposed Liberty Development Project. Borings were drilled at the site of a proposed production island and along three alternative marine pipeline routes. Two routes run south to connect onshore with the proposed Badami Pipeline and one route goes to the west to the existing Endicott facilities. The location of each drill hole is shown on Plate 1.

The geotechnical borings were drilled to depths of 11.0 to 112.5 feet below mudline, using a CME-75 drill rig equipped with hollow stem augers. Samples of the soil were obtained by pushing thin wall samplers and by driving split barrel samplers. The samples were tested in the laboratory for engineering properties. Closed pipe was left in four borings, and ground temperatures were measurements were attempted in March and April. In March, temperature measurements were recorded at two boreholes before weather prohibited further data collection. In April, temperature measurements were taken in all recoverable borings, however a malfunction of the thermistor string resulted in unusable data.

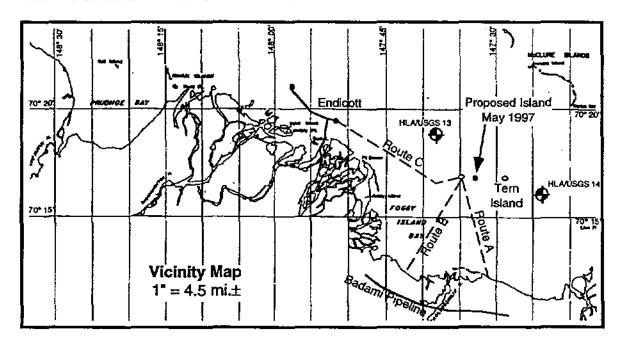
In Foggy Island Bay the offshore borings show Holocene soil to as deep as 18 feet below mudline but generally less than 9 feet deep and commonly missing. The Holocene soils include materials deposited in beach, delta, lagoon and shoal environments. The underlying Pleistocene soils include the marine Flaxman formation and deeper beach and alluvial sand and gravel.

Hard ice-bonded permafrost is present onshore and under the near shore, shallow waters. Ice-bonded permafrost is also found offshore where the stiff, overconsolidated Flaxman formation is present. Bonded permafrost was found at the island location and in one boring on the pipeline route to Endicott.

The soils are generally fine grained in the top ten feet and are commonly medium stiff to stiff except for occasional pockets of soft material where Holocene soils are present. The Holocene fine grained soils are compressible. The Pleistocene soils are heavily overconsolidated so they have small to moderate compressibility.

INTRODUCTION

This report presents the results of the geotechnical investigation conducted for the proposed Liberty Development Project, an offshore petroleum project in Foggy Island Bay. The proposed project will be located southeast of the Endicott Field and about 18 miles east of Deadhorse. The general location of the work is shown in the map below and details of the area are shown on Plate 1.



BP Exploration (Alaska), Inc., (BPXA) is evaluating the feasibility of developing the Liberty field. The potential petroleum field was explored in early 1997 by drilling a slant well from a spray ice island constructed at the remnant of Tern Island. Based on the findings from that exploration well and concurrent geophysical surveys, the proposed production island was relocated about 1/2 mile east of where we had done our drilling.

The oil will be transported from the production island in a steel pipeline. Offshore, the pipeline will be buried in the ocean floor; onshore, the pipeline is expected to be supported above grade on piling. The island and pipeline will be constructed using winter ice roads. Modules will be brought to the island during the ice free summer season. Emergency support and crew changes will be via helicopter.

The geotechnical exploration was started during the fall of 1996 when existing data were collected to allow for an assessment of expected geotechnical conditions. Also to



provide data for the design of the spray ice island, DM&A drilled one boring through the remnant of Tern Island, an abandoned gravel island about two miles to the east of the proposed Liberty Island. Using the existing data a field program was developed for exploring the offshore conditions at the Liberty project. The originally proposed island site and three alternative, offshore pipeline routes were explored in the spring of 1997 by drilling and sampling 30 borings and performing laboratory testing.

This winter's geotechnical work was performed in accordance with our contract with BP Exploration dated January 15, 1997. We coordinated our work with the Liberty Project Management Team through Mr. James C. Lewis of BPXA and Mr. José González Jáuregui of INTEC Engineering, Inc. Ms. Karen Wuestenfeld of BPXA provided permitting and environmental coordination. Field support and housing were provided by the Endicott Operating Unit.

The object of the geotechnical investigation was to explore the soil and permafrost conditions at the proposed production island site and along the three potential pipeline routes. This work was to include testing of the materials for pertinent engineering properties and to provide data for a conceptual design. The geotechnical work was divided into the following tasks:

- Drilling and sampling borings,
- Measuring ground temperatures,
- Primary and secondary laboratory testing in Anchorage, and
- * Data analysis and report preparation.

An independent, shallow sea floor evaluation of environmental conditions was performed by Montgomery Watson in conjunction with the startup of the field program.

EXPLORATION

Existing data

The geotechnical evaluation of the project area started with the collection and review of existing data. The surficial geology in Foggy Island Bay has been studied for many years by numerous agencies and at least one subsurface study was conducted by Shell Oil as part of their wildcat oil drilling program in the 1970s and 1980s.

The United States Geological Survey (USGS) has been involved in regional studies in the area since Schraeder first descended the Colville River right after the turn of the century. Leffingwell, who spent at least nine summers on the arctic coastal plain, produced the first definitive geologic study of the region and was the first to identify the geologic and ecological significance of the Flaxman boulder deposits. Surficial studies in the area have been conducted off and on since that time including detailed studies resulting from the Outer Continental Shelf Environmental Assessment Program (OCSEAP) of the 1970s. A complete narrative summary of government work in the area is presented in Public Data File 90-27 by the Alaska Division of Geological and Geophysical Surveys (Rawlinson, 1990).

In 1979 the USGS contracted with Harding Lawson Associates (HLA) to explore the geotechnical conditions in the proposed offshore lease sale area from the Kuparuk River to Flaxman Island. Two of the borings drilled during the spring of 1979 were located near the Liberty area (USGS/HLA 13 and USGS/HLA 14). The locations of those holes is indicated on the Vicinity Map in the Introduction.

Prior to building Tern Island, Shell Oil commissioned HLA to evaluate subsea soil conditions in the vicinity of the present island remnant. The results of the geotechnical investigation for the Tract 42 Well Site are in the HLA report dated February 1981 (HLA Job No. 9644,004.08). The island was instrumented by HLA after its construction and that data is presented in their report dated September 2, 1982 (HLA Job No. 9644,009.08).

A detailed geotechnical exploration was performed by HLA for the Endicott project in 1981 and 1982 and provides information to the west of the Liberty area and at the end of the Route C pipeline alignment.

Extensive sea floor studies also have been conducted to determine the extent of the "boulder patch" areas immediately northwest and northeast of the project site. These seafloor boulders are unique in the arctic and support an unusual abundance of sea life. In addition to the ecological significance of the boulder patch material, the boulders are an indicator of subsurface geology.

Selected references are cited at the end of this report.

Drilling

Between February 16 and 25, 1997, we drilled and sampled 30 borings. All work was conducted in accordance with the following permits:

Alaska Department of Natural Resources, Division of Land, Land Use Permit LAS 19505 (Tundra Travel) and LAS 19628,

Alaska Department of Fish and Game, Division of Habitat and Restoration FG96-III-0031, and North Slope Borough Administrative Approval NSB 96-052.

Four holes (Hole I-1 through I-4) were placed in the vicinity of the proposed production island, about two miles west of Tern Island. Ten holes (Hole A-1 through A-10) were placed along pipeline alternative "A" which runs SSE from the proposed production facility to tie in with the Badami Pipeline east of the Kadleroshilik (Kad) River Delta. Twelve holes (Hole B-1A and B-1 through B-11) were drilled along pipeline alternative "B" which runs SSW to join the Badami line on the west side of the Kad River Delta. An additional four holes were placed along pipeline alternative "C" which extends for about eight miles to the WNW to connect the Liberty production facility directly with the existing Endicott Complex. The location of each boring is shown on Plate 1 and a summary of locations, drilling information and conditions encountered at each boring are listed on Plate 2 and 3.

The coordinates of the proposed borings were calculated before the field work began and the hole location was established in the field using a Differential Global Positioning System (GPS). The GPS system employed by LCMF for field staking gave a precision for the initial hole locations at about +/-3 ft. The drilled locations with the exception of Hole A-2, were generally within 5 to 7 ft of the staked location. Hole A-2, near the beach line was moved 35 feet to the north to avoid a large snow drift. Hole B-1A was drilled about 50 feet southwest of Hole B-1.

The heavy equipment was mobilized along the Tern Island and Badami ice roads. Within the project area the existing ice roads and sea ice were used as much as possible so overland transport was minimized to the short access required to Holes A-1, B-1 and B-1A.

The drill was mounted on a sled, and the drill and operating area were protected from the weather by a framed enclosure. A second sled carried a survival shed and a generator. The drill equipment was supplied and operated by Discovery Drilling of Anchorage. Logistical support was provided by CATCO from their base of operations at Prudhoe Bay. A roller-driven Rolligon (CATCO RD-85) with 8 supporting air bags was used for transport. The drill rig and emergency shelter sleds were moved from site to site by the Rolligon. A 500-gallon fuel tank for day use was mounted on the Rolligon. When needed the RD-85 also served as crew carrier. Fuel supplies were replenished as needed by BP Liberty field contractors.

The crews were housed at BP's Endicott Facility and commuted to the project area by pick-up truck. Meals and office support were obtained at Endicott Operations Center. No on-site camp facilities were utilized except for the small sled-mounted emergency shelter that was kept with the drill rig at all times.

The work was performed on a double shift basis with crews working 12 hours at the drill rig. Each shift had a 4 person crew consisting of a DM&A geologist or engineer, a Discovery Drilling driller and a drill helper, and a CATCO operator. A DM&A technician provided expediting support as needed to both shifts and was responsible for sample packaging and shipping.

The borings were drilled to depths of 11.0 to 105 feet below the mudline using a CME-75 soils drill rig equipped with hollow stem augers. The soils were logged by Mr. Walt Phillips, PG, Ms. Erin Bashaw, EIT, and Mr. Mikal Hendee, EIT, as the borings were drilled. Samples of the soil were obtained by pushing thin wall samplers, by driving split barrel samplers and by grab sampling off the augers. Sampling was generally attempted at five-foot intervals.

In fine grained soils (silts and clays), thin-walled (Shelby) tubes were used where practical (this method is designated as Tw on the boring logs). The Shelby tubes are 3-inch OD (2.87-inch ID) and were pushed into the soil using the hydraulic down pressure of the drill rig. The sample was generally left in the tube, and the ends of the

tube were sealed. The Shelby tubes of unfrozen soil were protected from freezing and were transported in cushioned boxes. For sand and gravel or for fine grained soils that were too stiff for a Shelby tube, the sampling methods consisted of split-barrel samplers advanced into the soil below the tip of the auger by driving with an above ground hammer. The CME-75 was equipped with an automatic hammer system (samples designated as Sha on the boring logs). The hammer weighed 340 pounds and had a drop of 30 inches. The drive samples were obtained in a 3-inch OD by 2.5-inch ID split barrel. Blow counts for each 6-inch increment of the drive were recorded. Six-inch long brass tubes were commonly used in the 3-inch OD sampler, and the tubes were logged and capped for subsequent examination and testing.

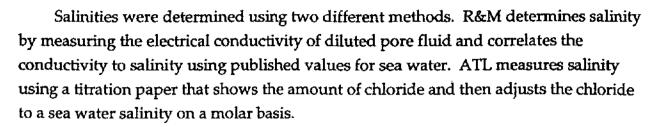
On shore, soils were sometimes sampled by grabbing cuttings off the auger flights (designated on the boring logs as Ag for auger grab). The ice content and soil type were logged and the samples were sealed in doubled plastic bags.

If the boring was in frozen soil that did not cave when the auger was pulled, the annulus between the PVC pipe and hole was backfilled with free flowing sand. The tube extends about 4-feet above the ground or ice surface and was marked by a plastic snow pole. The six-foot snow poles are bright orange and fitted with reflecting tape. Of the total of 30 test holes drilled, temperature monitoring pipe was left at 4 locations.

A graphic log of each borings drilled is presented in Appendix A on Plates A-1 through A-38. The soils and ice have been classified in accordance with the Unified Soil Classification System presented on Plate A-39. The boring logs show the type of sampler used and the blow counts required to drive the sampler the last 12 inches.

Laboratory testing

Laboratory testing in Anchorage included primary testing and secondary testing. Primary testing included moisture contents on most samples, salinity tests, moisture/density determinations on undisturbed samples, visual classification with torvane or pocket penetrometer strengths on undisturbed samples, and classification tests (sieve analysis and/or Atterberg Limits) and specific gravity tests. To provide information for corrosion design, samples were also tested for resistivity in accordance with the ASTM G-57 method. Laboratory testing was performed by two different laboratories: R&M Consultant, Inc., (R&M) and Alaska Testlab (ATL).



The results of the salinity and moisture contents are graphically shown on the boring logs and are tabulated on the Summary of Samples on Plates B-1 through B-11. The Summary of Samples also shows strength tests, thaw strain values, organic contents, specific gravity and classification test results. Results of the Atterberg Limit tests are shown graphically on Plate B-12, and the results of the particle size determinations are shown on Plates B-13 through B-21. The results of the Resistivity Tests are shown on Plate B-22.

Secondary testing was performed by ATL and included triaxial shear strength and consolidation tests of unfrozen samples and thaw consolidation tests of frozen samples. Both unconsolidated, undrained tests and consolidated, drained triaxial tests were performed. The results of the strength testing are summarized in the Summary of Samples in Appendix B. Laboratory data for the triaxial strength testing are presented in Appendix C. One dimensional consolidation tests were performed on unfrozen fine grain soils; the laboratory data sheets are presented in Appendix D.

Thaw strain tests were performed on 14 frozen samples; the laboratory data sheets are in Appendix E. The thaw strain samples were not extruded from the 2.47-inch diameter tubes but were tested in the tubes. In most cases the samples were close to six inches high. Working in a chest freezer, the end of the samples were hand trimmed to conform to the flat loading platens. Then the samples were loaded with a surcharge and allowed to thaw at room temperature. All but two of the tests were performed as single load tests. Step loads were applied to the other two samples after the thaw was completed so that a full consolidation curve could be developed. The data is discussed later in the report.

Ground temperatures

A 3/4-inch diameter pipe was installed in four borings to allow for the later measurement of ground temperatures. Multiconductor cables with thermistor beads mounted at various intervals were used to obtain ground temperature readings. For



each set of readings the thermistor cable was inserted in the pipe and left to stabilize for at least 45 minutes before the readings were obtained. A 4-1/2 digit multimeter was used to obtain the resistance of each thermistor.

In mid-March temperature readings were obtained at two onshore borings on Line "A" before weather prohibited further data collection. In April, temperature measurements were taken in all four pipes, however a malfunction of the thermistor string resulted in unusable data. We suspect that saltwater had gotten into the cable during the first set of readings in April. The measured temperatures obtained in March are tabulated and graphically presented in Appendix G.

REGIONAL SETTING

<u>Geology</u>

Foggy Island Bay is located on the northern edge of the physiographic province known as the Arctic Coastal Plain (Wharhaftig, 1965). The Arctic Coastal Plain is typified by gentle topography, ice bonded permafrost soils, wet tundra, wind-oriented thaw lakes, and braided and beaded stream channels. The coastal plain slopes gently northward to elevations of only a few feet above sea level at the shore of the Beaufort Sea.

The Foggy Island Bay shoreline is formed by a series of river deltas separated by an eroding coastline. The active delta of the east fork of the Sagavanirktok River forms the western edge of Foggy Island Bay. The Kadleroshilik and Shaviovik Rivers cross the coastal plain and discharge into the bay from the south. Tigvariak Island, an erosional remnant of onshore tundra, forms the eastern limits of Foggy Island Bay.

The ice-rich sediments beneath the onshore coastal plain subside significantly if they thaw (thermokarst collapse). Thermal erosion of the coastline occurs as wave action undercuts the ice-rich organic and silt deposits and blocks of tundra topped peat and icy silt collapse onto the beach (thermo-erosional niching). Thermal erosion is reported to about three feet per year between the Sagavanirktok River delta and Tigvariak Island (Hopkins et al., 1977). At a rate of three feet per year, archaeological sites which may have existed on an ancient coastline would have long ago fallen into the sea. In the surf zone, such remains would be subject to further destruction by wave and ice processes. Therefore, it is believed that even under ideal conditions, offshore archaeological remains would be totally or partially destroyed and difficult or impossible to recognize.

The offshore Holocene soils in Foggy Island Bay include beach, delta, lagoon, marine and shoal deposits, along with the Flaxman Lag. Pleistocene soils include the Flaxman Formation and marine, beach and alluvium. In the borings drilled for this exploration, the offshore Holocene sediments generally consisted of lagoonal and deltaic deposits (silt and organic silts) with some sandier beach and shoal deposits. The Holocene sediments are generally thin and are sometimes missing. The Flaxman Lag (the material that forms the "boulder patch" environments) was not found in the

borings, but this surface cobble and boulder material is best found using geophysical methods or by direct observation by divers.

The uppermost layer of the Pleistocene deposits is part of the Flaxman formation (part of the Gubic formation?), a marine unit probably deposited about 70,000 years ago. A beach deposit of sand and gravelly sand is found at the base of the marine unit and is in turn underlain by alluvial sand and gravel deposited as glacial outwash.

Where the Pleistocene marine deposits outcrop on the sea floor, they are often composed of overconsolidated, stiff to very stiff silt or clay. At Tigvariak Island and along other parts of the coast, scattered gravel, cobbles and boulders are incorporated within the marine deposits. The boulders, commonly called Flaxman boulders, are characterized by mineral constituents foreign to the geology of Alaska. Some of the rock has been traced to bedrock in the Canadian shield. It is believed that the Flaxman boulders represent glacial dropstones which were ice rafted to their present locations during an interglacial period of elevated sea level. As the grounded ice bergs rotted in place, the cobbles and boulders dropped to the seafloor and were then buried in fine grained marine deposits. When the fine-grained marine sediments are eroded away, the coarse material of the Flaxman formation remains as a lag deposit of cobbles and boulders and forms the "boulder patch" environments.

Climate

Foggy Island Bay is within the Arctic Climatic Zone and has an arctic maritime climate. At Prudhoe Bay temperatures range from a mean of 7°C in July to a mean of -30°C in February. The mean annual temperature is about -13°C.

Total annual precipitation ranges from about 10 and 22 cm. Of this, as much as half is rainfall which occurs between June and September. Snowfall is generally greatest in October.

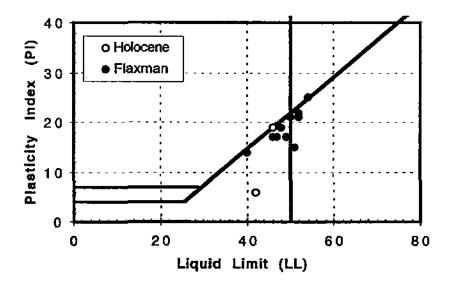
Winds on the coastal plain are generally from the east and northeast in summer and from the west and northwest in Winter. Westerly summer winds, generally associated with storms, are sometimes responsible for surge water that may inundate low-lying areas.

SOIL AND PERMAFROST CONDITIONS

Soil Conditions

The borings show soil conditions that fit with the previously discussed geologic depositions. The conditions at the island site and along the three pipeline alternatives are shown on Plates 4 through 7. Away from the beach line, most of the near sea floor soils consist of silts and clays and silty sand. As expected for lagoonal soils, the near surface deposits frequently contain a considerable amount of organic matter. Pieces of peat and organic silt layers are also encountered.

The Atterberg Limit tests generally show the fine grained materials vary widely in plasticity, from non plastic silt (ML) to plastic clays (CH) and plastic silt (MH). The more plastic material is generally associated with the Flaxman formation. Three other fine-grained samples that tested non-plastic were all from the Holocene sediments.



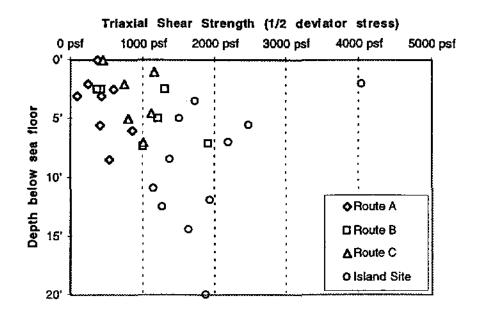
Uniaxial consolidation tests were performed on 10 samples from the island site. The tested samples of Holocene material show a modified compression index of 0.04 to 0.27, with the lower value associated with a silty sand. The overconsolidated materials associated with the Pleistocene Flaxman formation show a modified recompression index of 0.01 to 0.04. The results of the consolidation tests are summarized in the following table.

Summary of Consolidation Tests

Boring	Depth	Soil Type	Geology	Moisture	Density	Cc	Cr
I-1	3.0'	ML+OL	Holocene	50.9%	65 pcf	0.27	0.08
I-1	5.5'	ML	Pleistocene	29.6%	93 pcf	0.12	0.04
I-1	15.5'	MH	Pleistocene	35.0%	85 pcf	0.17	0.04
I-2	9. 7 '	ML	Pleistocene	28.6%	99 pcf	0.18	0.06
I-3	2.5'	ML	Holocene	28.3%	97 pcf	0.08	0.02
I-3	7.5'	МH	Pleistocene	32.4%	91 pcf	0.12	0.03
I-3	12.5'	ML	Pleistocene	36.2%	86 pcf	0.20	0.04
I-3	20.0'	ML	Pleistocene	28.2%	94 pcf	0.11	0.02
I-4	2.0	SM	Holocene	32.0%	95 pcf	0.04	0.01
I-4	22.0'	MH	Pleistocene	28.7%	90 pcf	0.13	0.02

If a gravel island with 20 feet of freeboard is constructed over a Holocene layer that is six feet thick with an underlying Pleistocene layer that extends to a depth of 25 feet, and using the higher values for virgin compression ratio (Cc = 0.25) in Holocene layer and the rebound value (Cr = 0.04) for the stiffer underlying material, a total settlement due to consolidation of the sediments would be about three feet. Depending on the internal drainage conditions (presence of sand lenses), the settlement would take six months to two years to be essentially complete.

Much of the fine grained soil is overconsolidated with shear strengths greater than 1000 psf. The weakest materials are in the top five feet and found along the north-south pipeline routes (Routes A and B). The soils at the island site are stiff to very stiff.





Bonded Permafrost

Permafrost is defined as a material that remains below 32° F for two or more years. Except for a thin layer near the sea floor, all of the offshore soils remain below 32°F. However, for engineering purposes, the critical issue is whether the material is bonded with ice. Because of variations in salt content and the associated freezing point depression, temperature alone does not define the engineering characteristics of the material. As expected, ice bonded permafrost was observed in the onshore borings and beach borings (A-1, A-2, B-1, B-1A and B-2). The onshore area contains massive ice as polygonal wedges.

Offshore, the surface of the bonded permafrost appears to drop away steeply. No bonded material was observed either Boring A-3 or B-3 drilled just inside the 6-foot isobath. Both borings were drilled to depths greater than 30 feet below mudline.

Bonded permafrost was found offshore in deeper waters in Boring C-3 (11 feet of water) and at the island, Borings I-1 and I-4 (about 20 feet of water). This permafrost under deeper water is believed to be related to the presence of the Flaxman formation silt and clay. The silt and clay are heavily overconsolidated and have a low permeability. Consequently, the diffusion of seawater salts into the material is slower than for silty sand or sand materials. The intrusion of salt is what depresses the thaw point of the material and leads to a loss of ice bonding. Similar ice bonded conditions were found in the HLA/USGS Boring 13 and at many locations at the Endicott site.

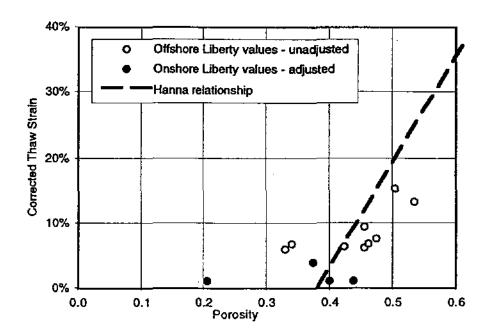
Thaw Strains

All of the samples tested for thaw strain were also tested for density, moisture content and pore water salinity, and typical samples were tested for classification. The following table shows the pertinent data from the laboratory testing. Many hammer blows (N-value) were needed during the drive sampling of the tested samples from the onshore and near shore borings (B-1, B-1A and B-2). The measured dry density and moisture contents indicate that the samples are under-saturated. This is not reasonable. Therefore, we have corrected the dry densities to values corresponding to ice saturation for the measured moisture content, and we have also adjusted the measured thaw strain to correct for the internal fracturing present before testing. Values of thaw strain were not reduced below 1%. The sample from Boring I-1 at 46 feet appears to have an incorrect moisture content so we have not included it in the plot of data.

						Uncor	rected La	b Values	1	Adjusted Values 1		
Bor. No.	Dept	Salt	USCS	N- value	Moist Cont	Lab Dry Dens.	Lab Porosity	Thaw Strain	Ice Sat.	Sat. Dry Dens.	Thaw Strain	S≃100% Porosity
''''	41			Adide	Conta	Data.	10105119	Juan		Derb.	Juan	Lotonly
B-1	11.0	3 ppt	SM	48	27.0%	71 pcf	0.571	14.40%	59%	93 pcf	1.00%	0.439
B-1	14.5'	2 ppt	SP-SM	54	23.0%	83 pcf	0.499	11.30%	67%	99 pcf	1.00%	0.400
B-1A	14.0'	3 ppt	SM	100	20.7%	85 pcf	0.486	25.50%	63%	103 pcf	3.77%	0.375
B-2	10.1	56 ppt	SM	90	9.0%	124 pcf	0.250	4.30%	78%	131 pcf	1.00%	0.207
C-3	20.0	21 ppt	ML	ø	38.6%	77 pcf	0.534	13.10%	97%	78 pcf	11.59%	0.528
. C-3	20.5	3 ppt	ML	8	35.9%	82 pcf	0.504	15.30%	102%	81 pcf	16.32%	0.510
C-3	25.5'	0 ppt	SM	0	25.7%	90 pcf	0.456	6.00%	89%	95 pcf	0.57%	0.427
C-3	26.0'	0 ppt	SM	34	27.7%	87 pcf	0.474	7.50%	89%	92 pcf	1.93%	0.445
C-3	30.8	5 ppt	SM	55	27.7%	89 pcf	0.462	6.70%	94%	92 pcf	3.51%	0.445
I-1	45.5'	2 ppt	SP-SM	110	25.1%	90 pcf	0.456	9.30%	87%	96 pcf	2.81%	0.421
I-1	46.0'	1 ppt	SP-SM		34.7%	94 pcf	0.432	8.10%	133%			
I-1	65.5	10 ppt	SM	160	16.5%	111 pcf	0.329	5.90%	98%	112 pcf	5.03%	0.323
I-1	66.0"	6 ppt	SM		17.3%	109 pcf	0.341	6.70%	97%	110 pcf	5.56%	0.334
I-4	52.5	2 ppt	SM	93	24.1%	95 pcf	0.424	6.30%	95%	97 pcf	4.02%	0.411

Bold values in dashed boxes are plotted in the following chart

These adjustments were not applied to the offshore samples where the measured densities and moisture contents are closer to total saturation. The potential corrections are shown in the preceding table and range between the uncorrected and adjusted values probably reflects the range of error in the testing. The following chart shows the thaw strain data versus the porosity of the sample and compares the data with the relationship developed by Hanna for silts and fine sands.



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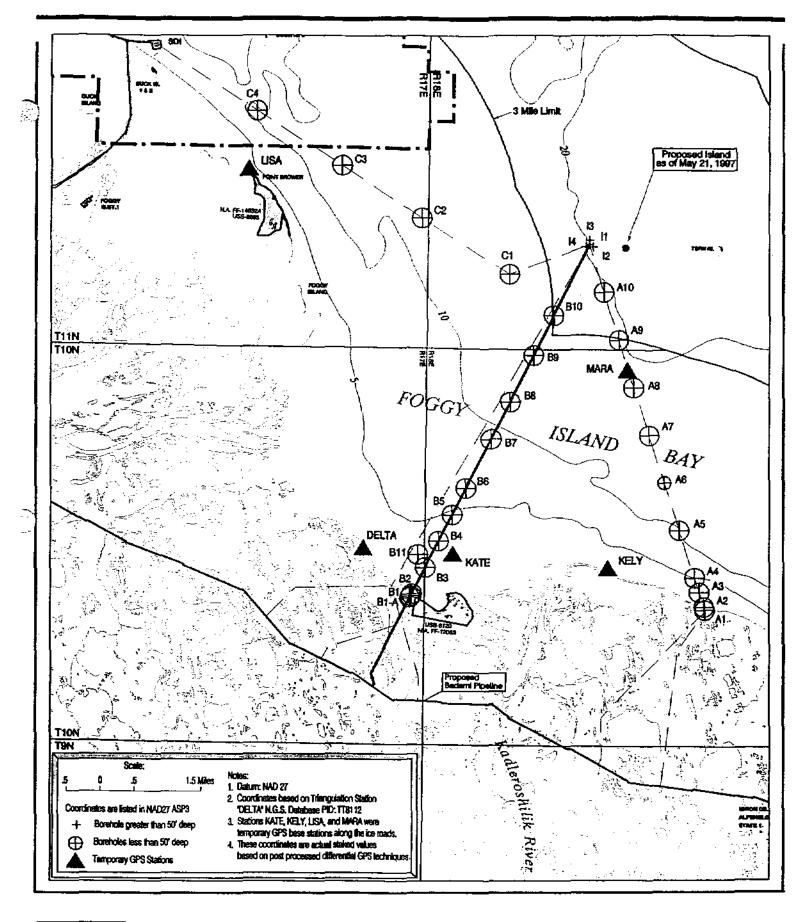
Harding Lawson Associates; February 9, 1981; Geotechnical Investigation, Tract 42 Well Site, Beaufort Sea, Alaska; for Shell Oil Company

Harding Lawson Assoc.; 1979; USGS Geotechnical Investigation, Beaufort Sea, Alaska

Harding Lawson Associates; January 1982; Geotechnical Engineering Considerations, Duck Island Development Project, Beaufort Sea, Alaska; for Exxon Company USA

Reimnitz, E., and Kempema, Ross and Minkler; 1980; Overconsolidated surficial deposits on the Beaufort Sea Shelf, USGS Open File Report 80-draft

ILLUSTRATIONS





Duane Miller & Associates

Arctic & Geotechnical Engineering Job No.: 4119.22

Date: June 1997

Map File No. ip13373a.dgn

BORING LOCATIONS

Liberty Development Beaufort Sea, Alaska PLATE

1

Summary of berty Borings

Bor. No.	As Drilled ASP Zone 3 Northing	As Drilled ASP Zone 3 Easting	Elev.*	Date/Time Finished	Water Depth	Ice Thick- ness	Boring Depth*	Bonded Perma- frost	PVC Pipe	Base of Holocene Lagoon	Base of Silt	Top of Sand/ Gravel	Comments
A-1	5,925,145 ft	313,256 ft	7.0'	2/16 18:30	ne	ne	30.0'	yes	30'	ne	9'	9,	onshore tundra
A-2	5,925,385 ft	313,200 ft	-3.6'	2/18 0:00	ne	3.6	31.5'	yes	none	5'	5'	5'	
A-3	5,926,564 ft	312,842 ft	-5.9'	2/19 18:30	5.9'	3.5'	31.5'	no	30'	ne	ne	1 7 '	shoal sand to 17'
A-4	5,927,743 ft	312,496 ft	-6.2'	2/20 1:45	6.2	4.0'	32.8'	no	none	3.5'	3.5'	5'	
A-5	5,931,558 ft	311,346 ft	-10.8'	2/20 6:30	10.8'	3.5'	28.2'	no	none	10'	10'	10'	interbedded silt and sand to 10'
A-6	5,935,382 ft	310,204 ft	-17.7	2/20 13:30	17.7'	3.9'	29.5'	no	none	7'	<i>7</i> '	16'	Silty sand from 7' to 16'
A-7	5,939,201 ft	309,070 ft	-19.3	2/17 1:00	19.3'	4.2'	30.0	no	none	11.5'	9.5'	14'	
A-8	5,943,021 ft	307,941 ft	-19.3'	2/18 11:00	19.3'	4.3'	37.5'	no	none	10'	10'	27'	Silty sand from 10' to 27'
A-9	5,946,839 ft	306,798 ft	-19.8'	2/18 16:30	19.8'	4.2'	33.5'	no	none	10'	19'	22.5'	Silty sand from 19' to 22.5'
A-10	5,950,658 ft	305,654 ft	-19.0	2/21 22:05	19.0'	4.0'	34.5'	no	none	11'	23'	23'	
B-1	5,926,747 ft	289,876 ft	7.0'	2/19 5:00	ne	ne	30.0	yes	none	ne	ne	12'	Tundra organics to 4', Ice wedge below
B-1A	5,926,712 ft	289,841 ft	7.0'	2/25 6:30	ne	ne	30.0'	yes	none	ne	ne	15'	Peat to 0.5' over Silty sand
B-2	5,926,908 ft	289,968 ft	0.0	2/19 1:15	ne	1.3'	31.2'	yes	none	ne	пе	1.5'	
B-3	5,928,987 ft	291,067 ft	-5.6'	2/18 21:45	5.6'	4.3'	36.2'	no	none	11.5'	9.5'	11.5'	Silt layer from 7' to 9.5'
B-4	5,931,094 ft	292,175 ft	<i>-5.7</i> '	2/18 18:00	5.7'	3.8'	22.0'	no	none	7'	7	10'	
B-5	5,933,178 ft	293,273 ft	-6.7'	2/20 18:00	6.7	4.0'	26.5	no	none	7.5'	7.5'	10.5	

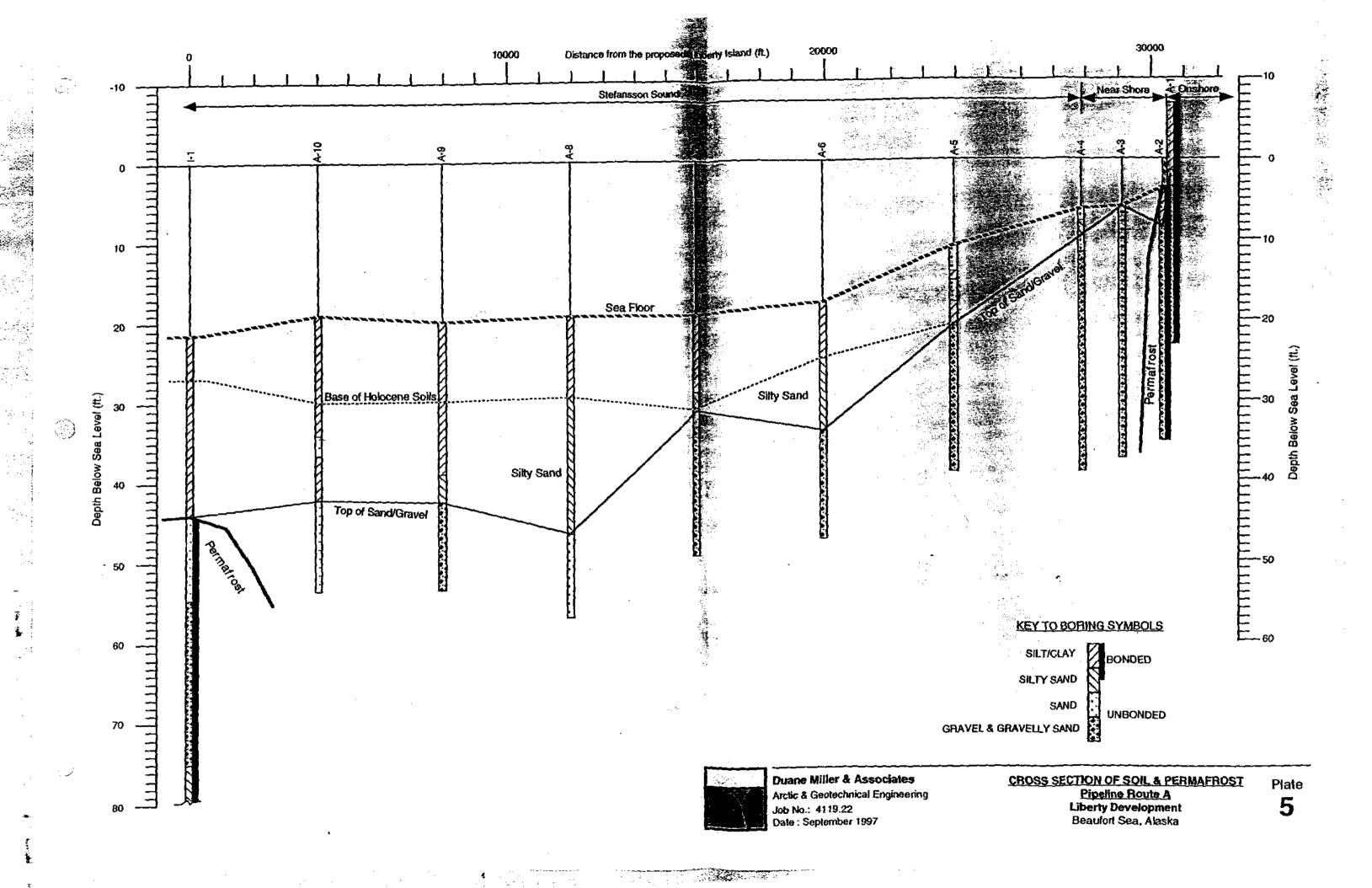
Summary of Loerty Borings

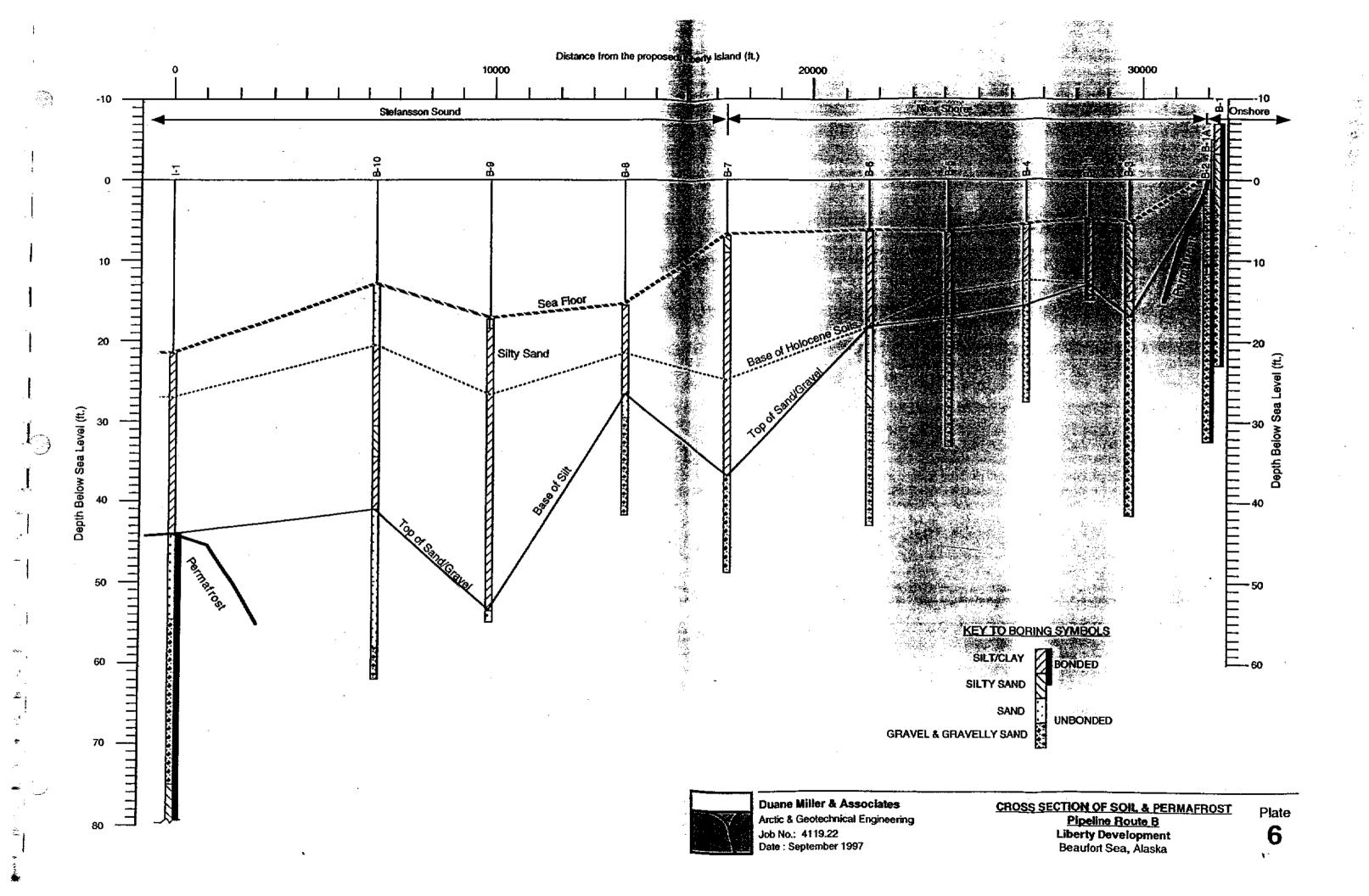
Bor. No.	As Drilled ASP Zone 3 Northing	As Drilled ASP Zone 3 Easting	Elev.*	Date/Time Finished	Water Depth	Ice Thick- ness	Boring Depth*	Bonded Perma- frost	PVC Pipe	Base of Holocene Lagoon	Base of Silt	Top of Sand/ Gravel	Comments
B-6	5,935,285 ft	294,379 ft	-6.5'	2/20 0:10	6.5'	3.8'	36.5'	no	none	12'	12'	12'	
B-7	5,939,157 ft	296,424 ft	-7.1'	2/21 5:30	7.1'	4.2'	41.6'	no	none	18'	30'	30'	Peat layer from 16.5' to 17.5'
B-8	5,942,084 ft	297,978 ft	-15.5'	2/18 2:00	15.5'	3.8'	27.0'	no	none	6' '	11'	11'	
B-9	5,945,758 ft	299,909 ft	-17.3'	2/18 4:00	17.3'	4.0'	49.6'	no	none	9.5'	36'	36'	
B-10	5,948,920 ft	301,578 ft	-13.0	2/21 14:30	13.0	4.0*	49.0'	no	none	7.5'	28'	28'	Sand from mudline to 7.5'
B-11	5,930,040 ft	291,621 ft	-5.0'	2/19 9:30	5.0'	4.0'	10.0'	no	none	8'	ne	8'	between B-3 and B-4
C-1	5,952,264 ft	298,091 ft	-14.8'	2/25 1:30	14.8'	4.3'	29.5'	no	none	3.5'	вон	πe	Sand from mudline to 3.5'
C-2	5,956,813 ft	291,289 ft	-15.2'	2/24 22:30	15.2'	4.2'	29.0'	no	none	0'	вон	ne	
C-3	5,961,067 ft	284,970 ft	-11.3'	2/24 17:50	11.3'	4.5'	31.5	some	32'	0.	30'	30'	
C-4	5,965,516 ft	278,327 ft	-10.8'	2/24 11:30	10.8'	4.4'	27.0'	no	none	3.5'	20'	20'	Sand and Sandy Silt below 3.5'
I-1	5,954,484 ft	304,526 ft	-21.5'	2/23 5:30	21.5	3.5'	105.0	some	105'	5.5'	22.5'	22.5'	
I-2	5,954,304 ft	304,821 ft	-21.7	2/22 5:30	21.7'	4.8'	50.5'	no	none	7.5'	24'	24'	
1-3	5,954,846 ft	304,516 ft	-21.1	2/23 18:30	21.1'	4.0'	51.5'	no	none	3.5'	23'	23'	
I-4	5,954,317 ft	304,216 ft	-19.7	2/24 0:30	19.7'	4.2'	53.5'	yes	no	7'	23'	23'	Bonded below 44.5'

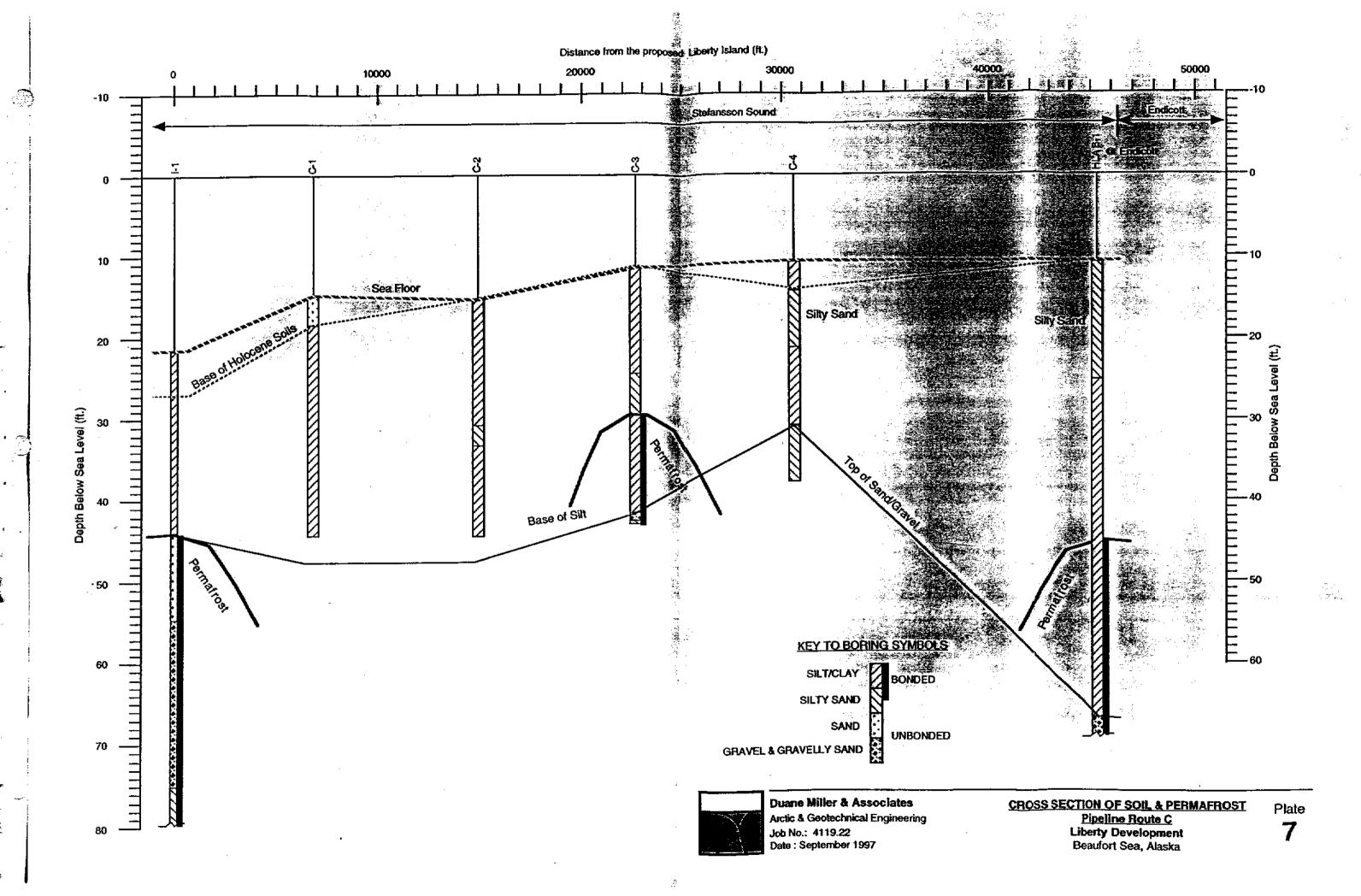
^{*} Boring depths are from mudline. Elevations assume water level = MSL

ne = none encountered, BOH is Bottom of Hole

Job No.: 4119.22 Date: September 1997







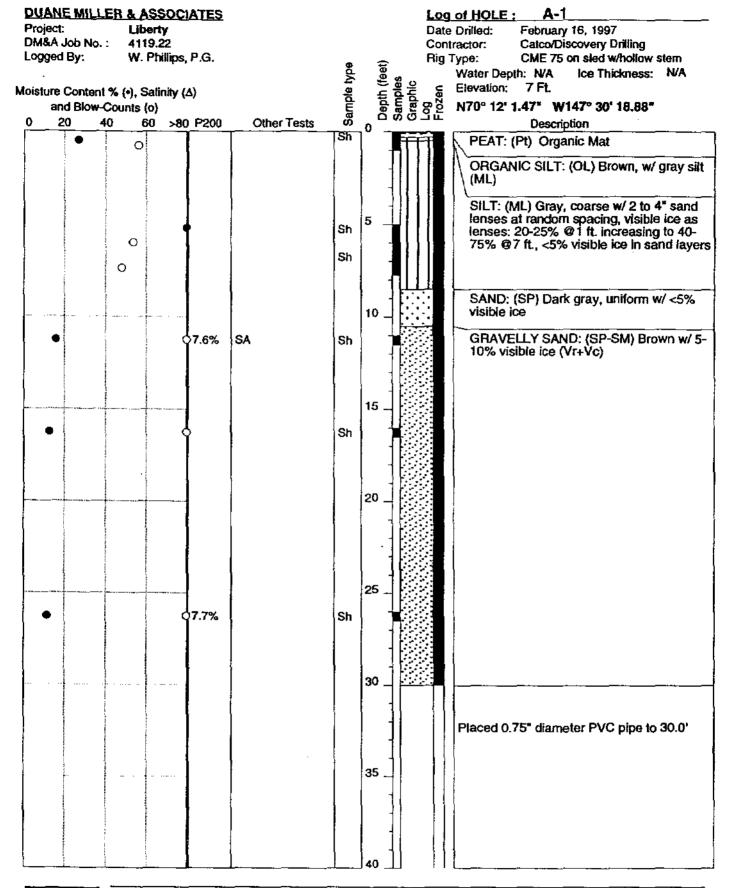
APPENDIX A

BORING LOGS and KEY TO DATA

Logs of Borings

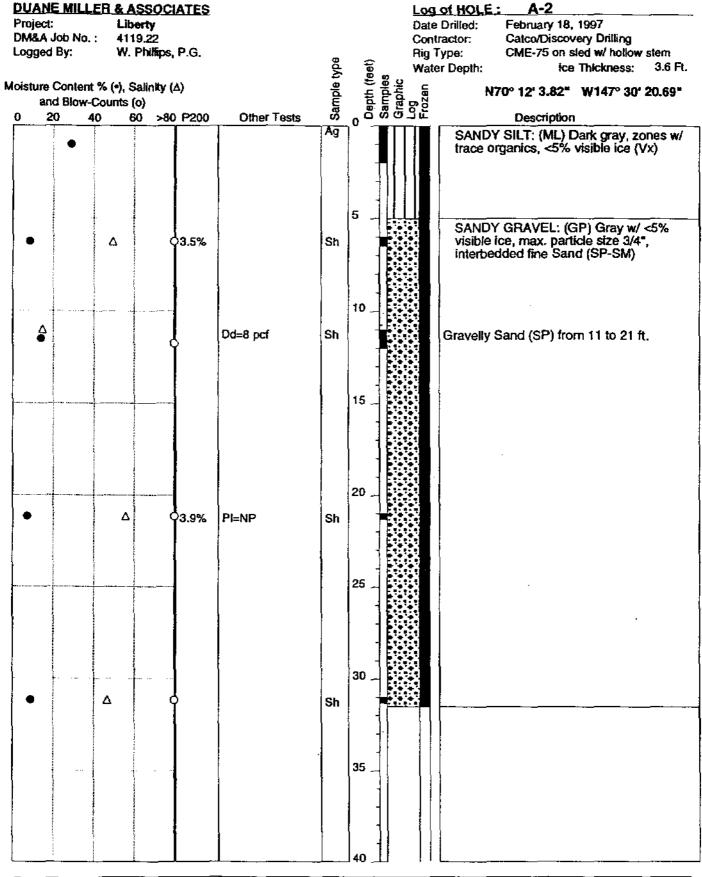
Plates A-1 through A-38.

Soil Classification System and Key to Data



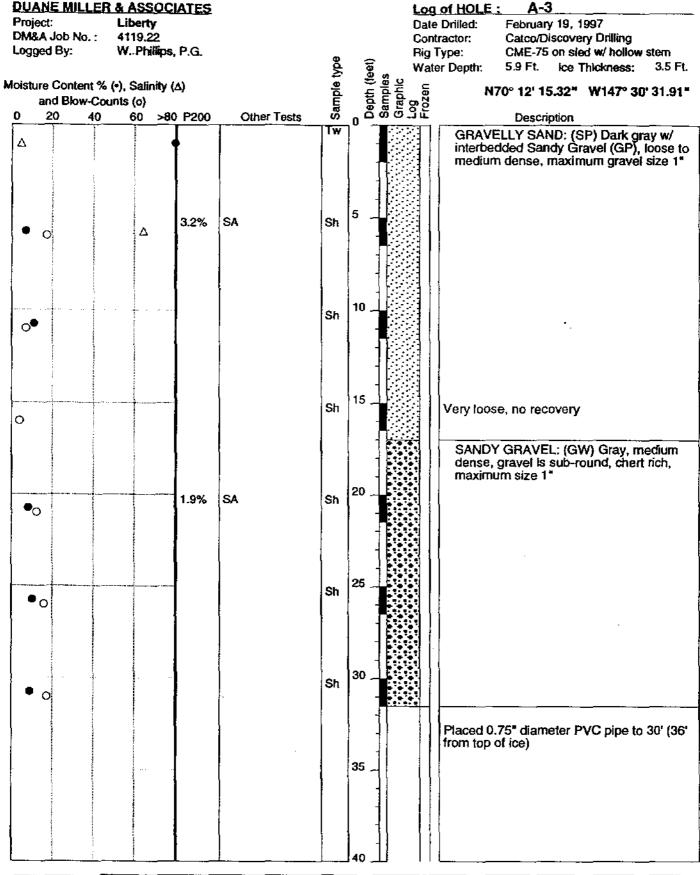


Job No.: 4119.22 Date: June 1997 LOG of BORING A-1 Liberty Development Beaufort Sea, Alaska





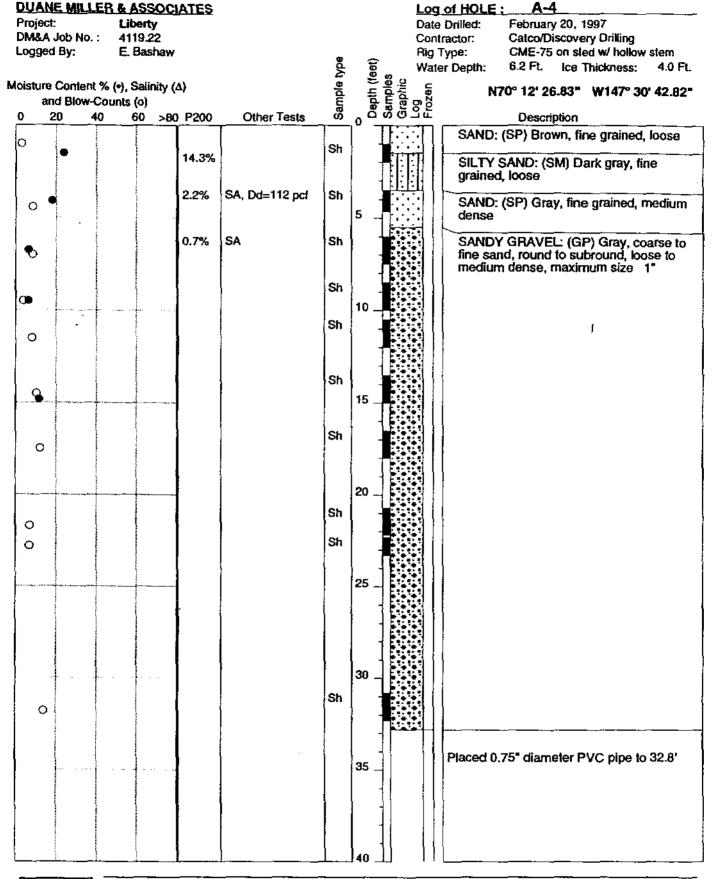
Job No.: 4119.22 Date: June 1997 LOG of BORING A-2 Liberty Development Beaufort Sea, Alaska





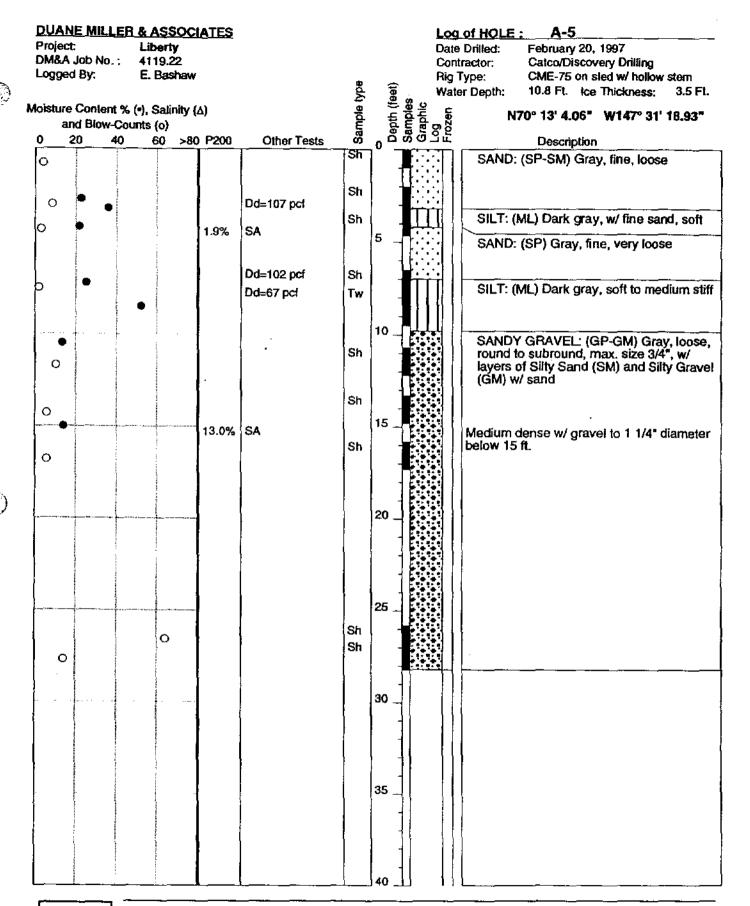
Duane Miller & Associates Arctic & Geotechnical Engineering

Job No.: 4119.22 Date: June 1997 LOG of BORING A-3 Liberty Development Beaufort Sea, Alaska



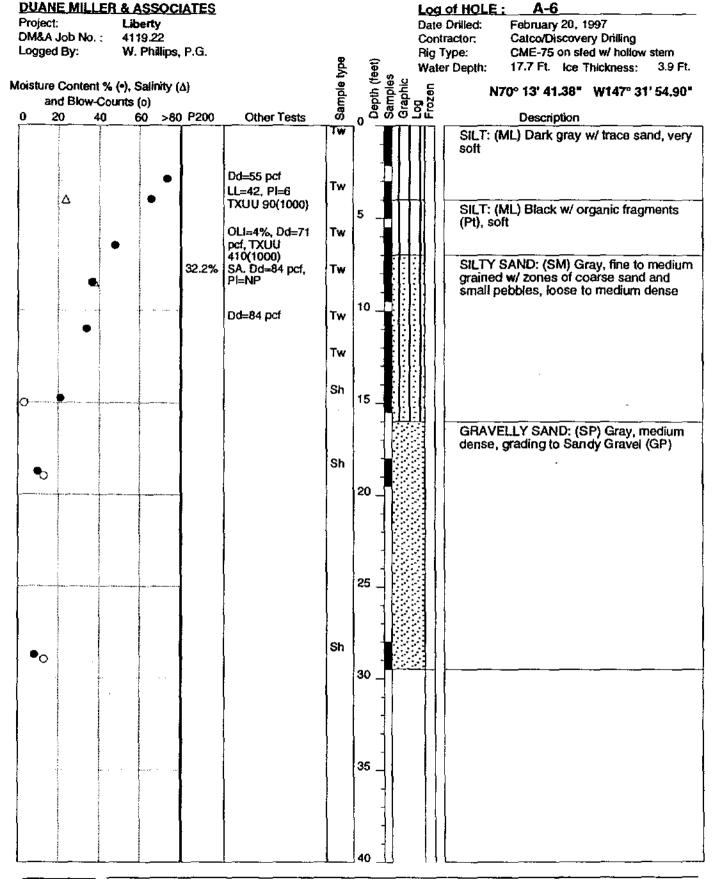


Job No.: 4119.22 Date: June 1997 LOG of BORING A-4 Liberty Development Beaufort Sea, Alaska



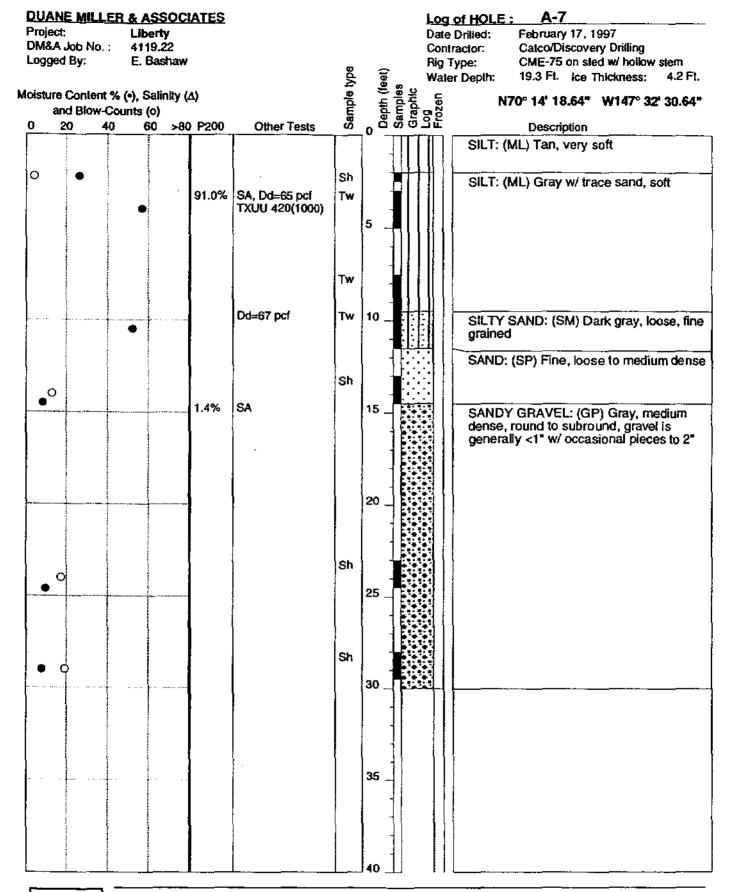


Job No.: 4119.22 Date: June 1997 LOG of BORING A-5 Liberty Development Beaufort Sea, Alaska



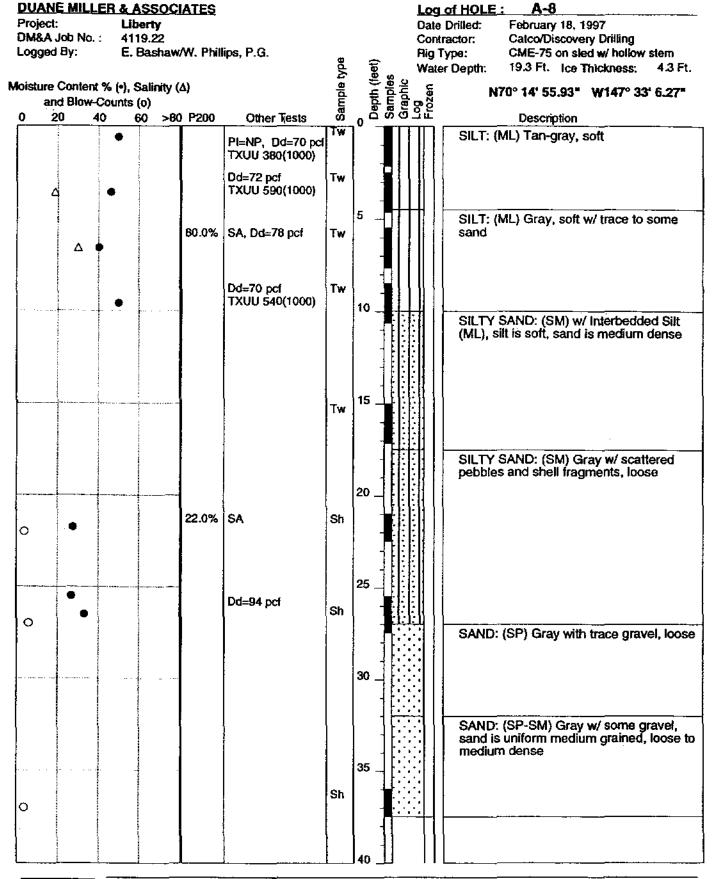


Job No.: 4119.22 Date: June 1997 LOG of BORING A-6. Liberty Development Beaufort Sea, Alaska



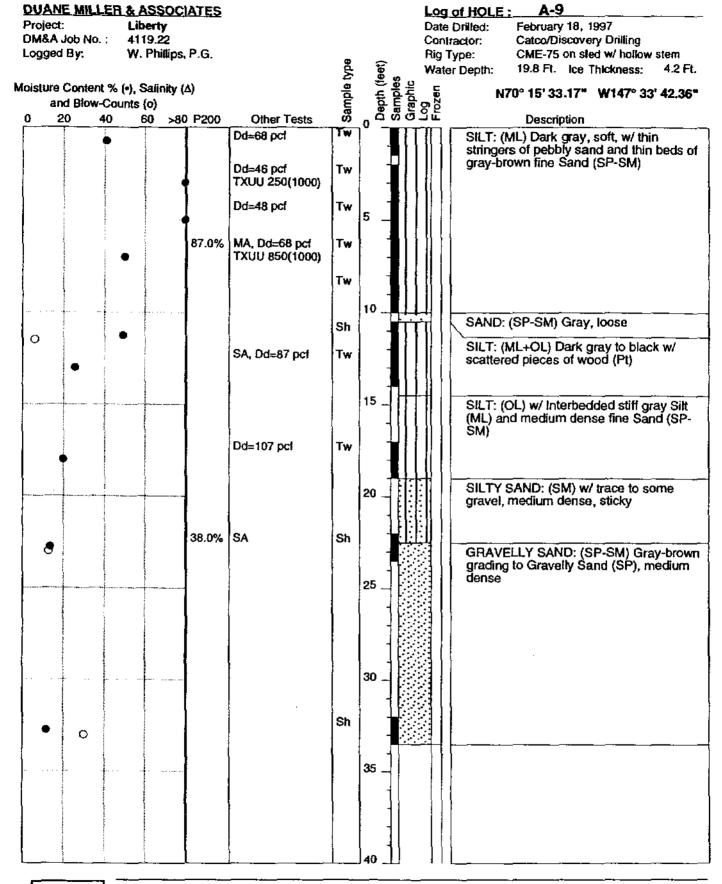


Job No.: 4119.22 Date: June 1997 LOG of BORING A-7 Liberty Development Beaufort Sea, Alaska



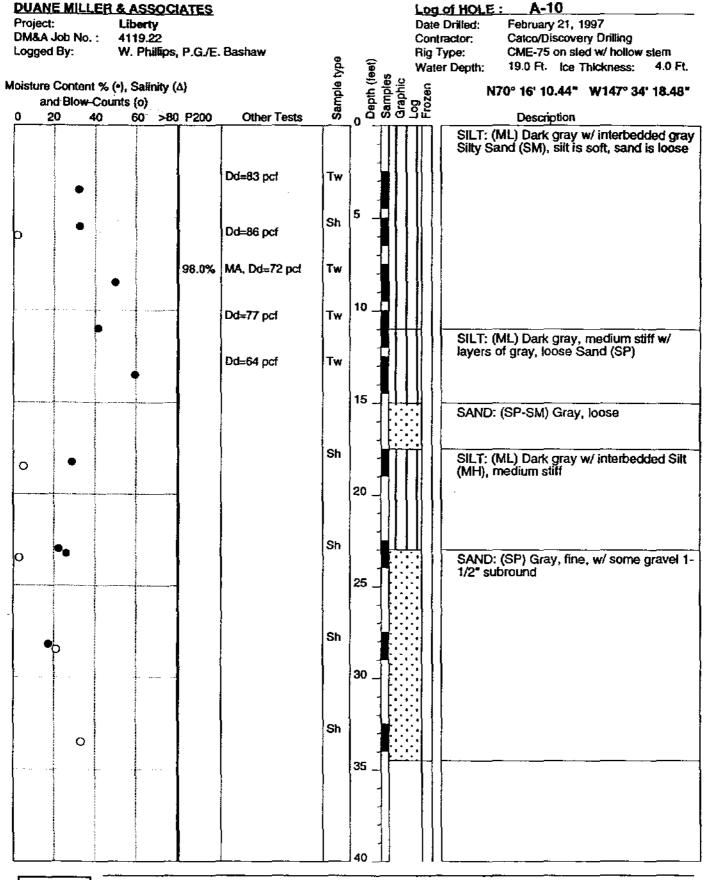


Job No.: 4119.22 Date: June 1997 Liberty Development Beaufort Sea, Alaska



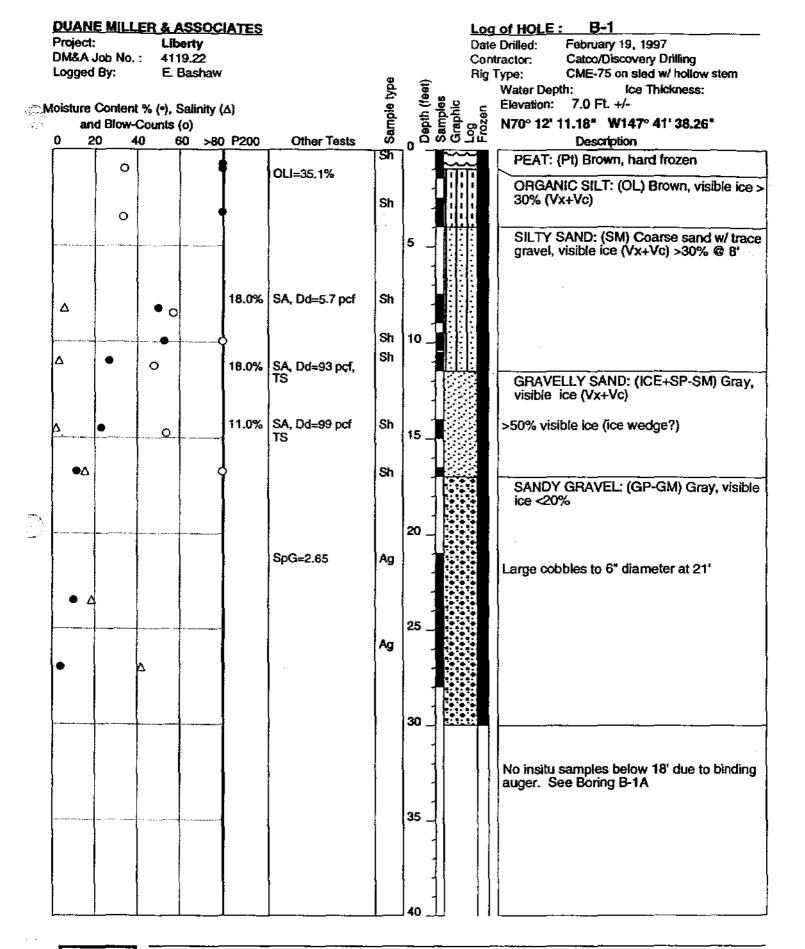


Job No.: 4119.22 Date: June 1997 LOG of BORING A-9 Liberty Development Beaufort Sea, Alaska



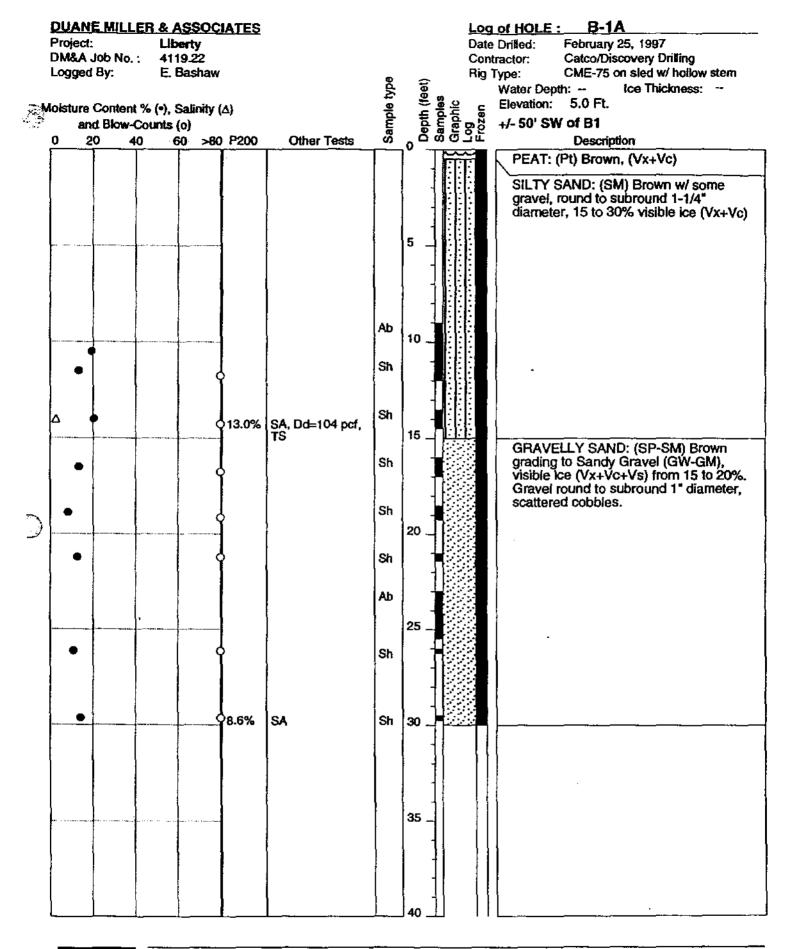


Job No.: 4119.22 Date: June 1997 LOG of BORING A-10 Liberty Development Beaufort Sea, Alaska



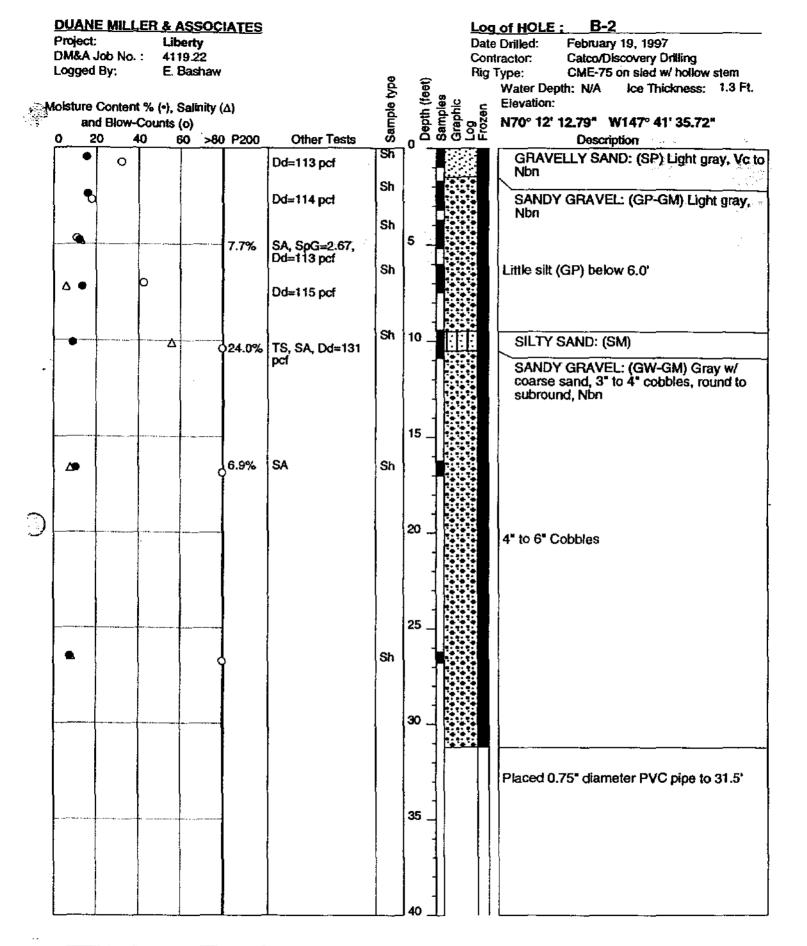


Job No.: 4119.22 Date: September 1997 Liberty Development Beautort Sea, Alaska



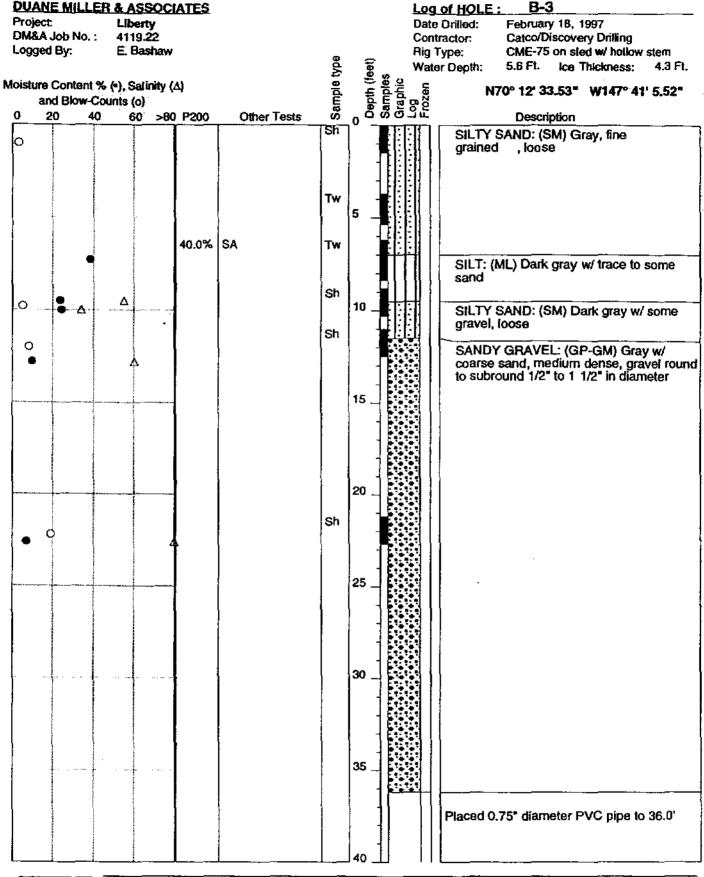


Job No.: 4119.22 Date: September 1997 LOG of BORING B-1A Liberty Development Beaufort Sea, Alaska



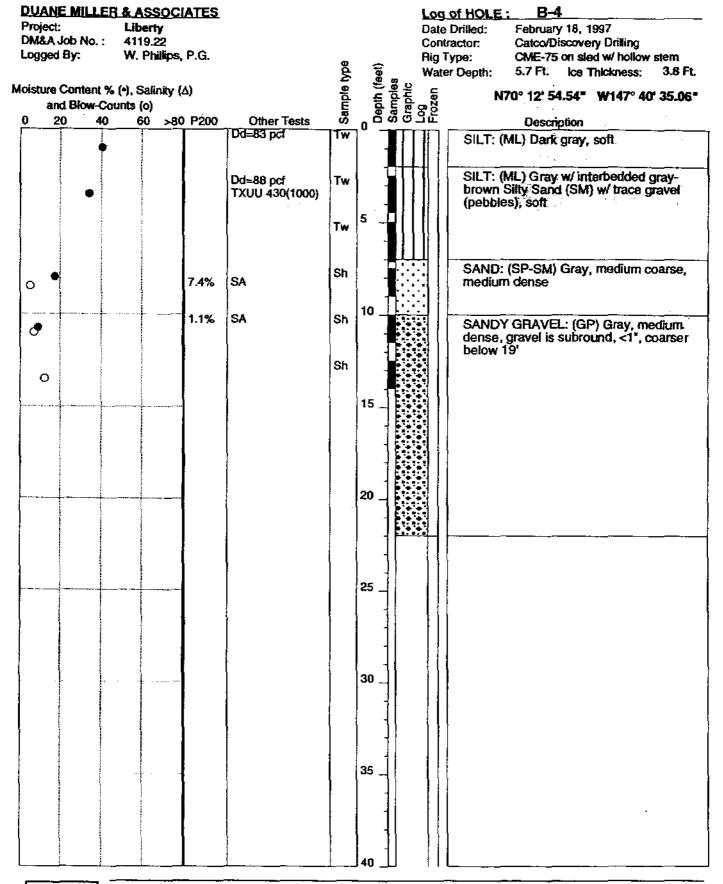


Job No.: 4119.22 Date: September 1997 Liberty Development Beaufort Sea, Alaska



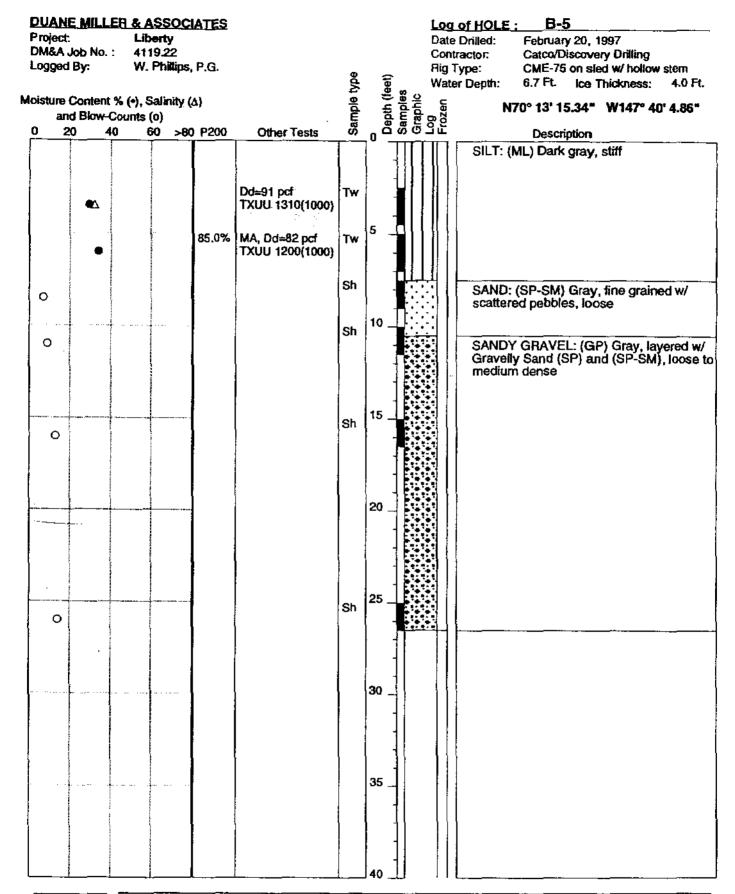


Job No.: 4119.22 Date: June 1997 LOG of BORING B-3 Liberty Development Beaufort Sea, Alaska





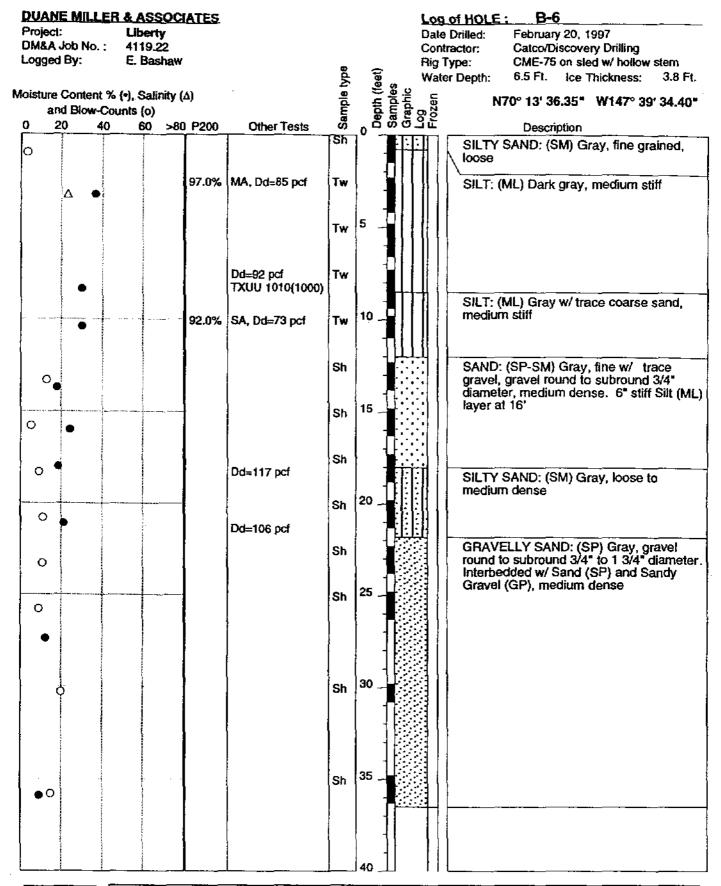
Job No.: 4119.22 Date: June 1997 LOG of BORING B-4 Liberty Development Beaufort Sea, Alaska





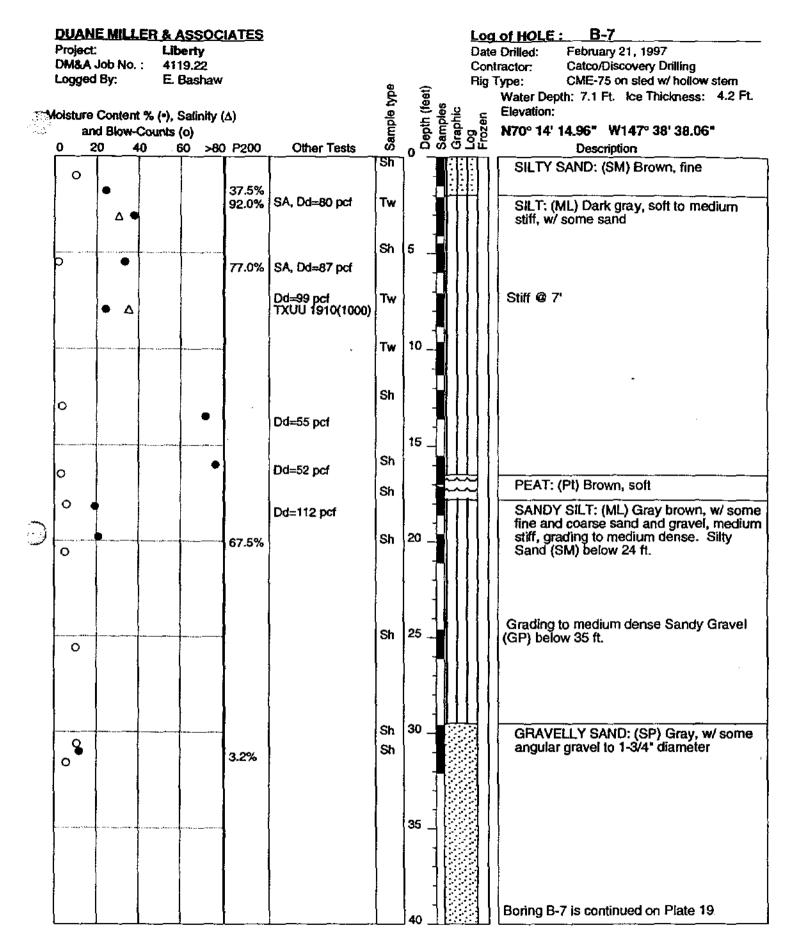
Duane Miller & Associates
Arctic & Geotechnical Engineering
Job No.: 4119 22

Job No.: 4119.22 Date: June 1997 LOG of BORING B-5 Liberty Development Beaufort Sea, Alaska



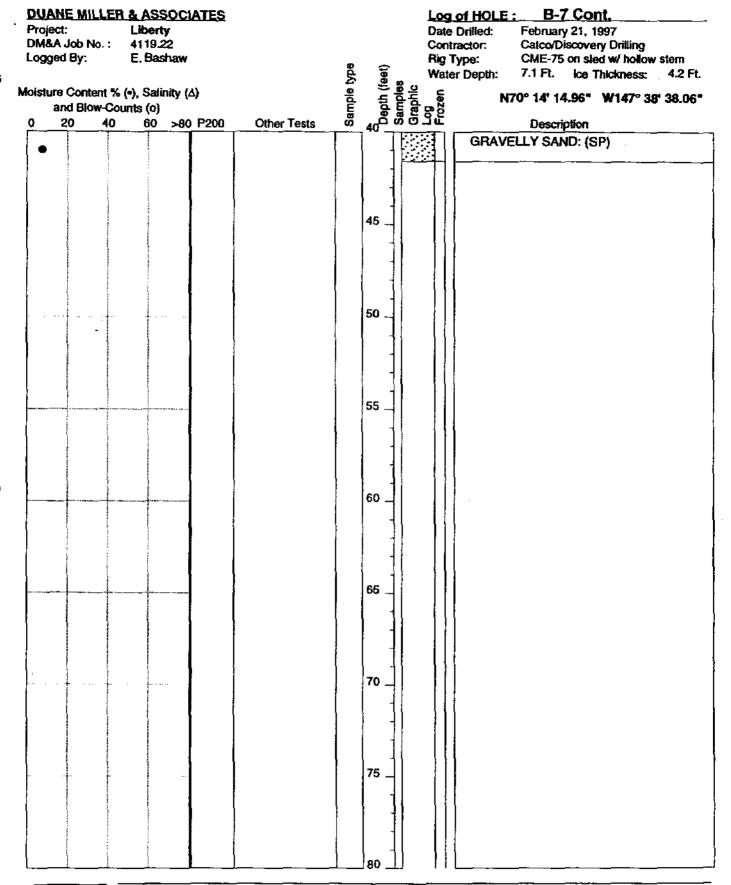


Job No.: 4119.22 Date: June 1997 LOG of BORING B-6 Liberty Development Beaufort Sea, Alaska



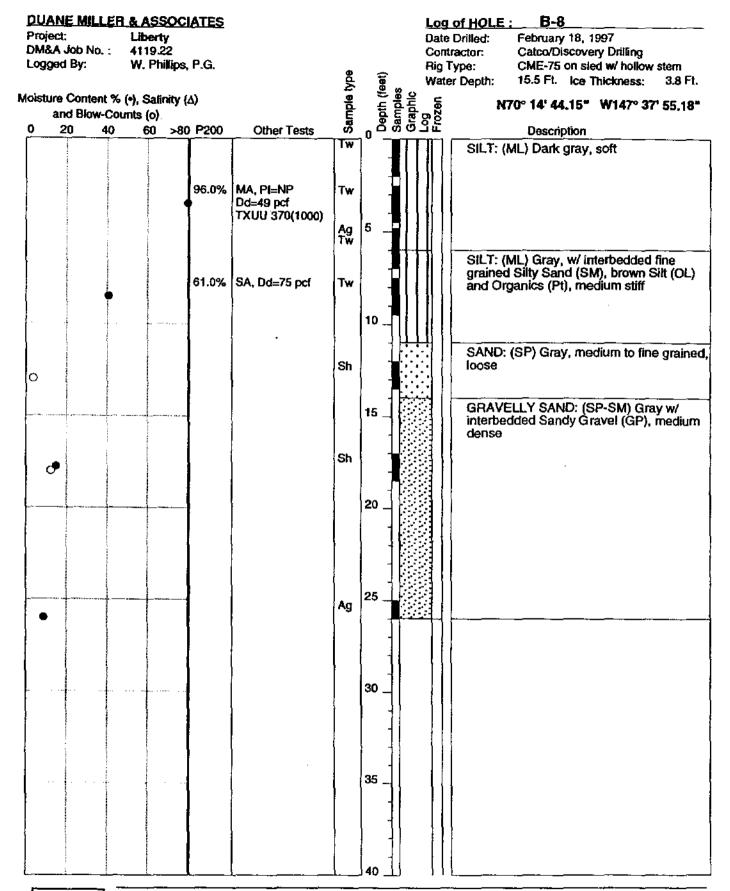


Job No.: 4119.22 Date: June 1997 Liberty Development Beautort Sea, Alaska





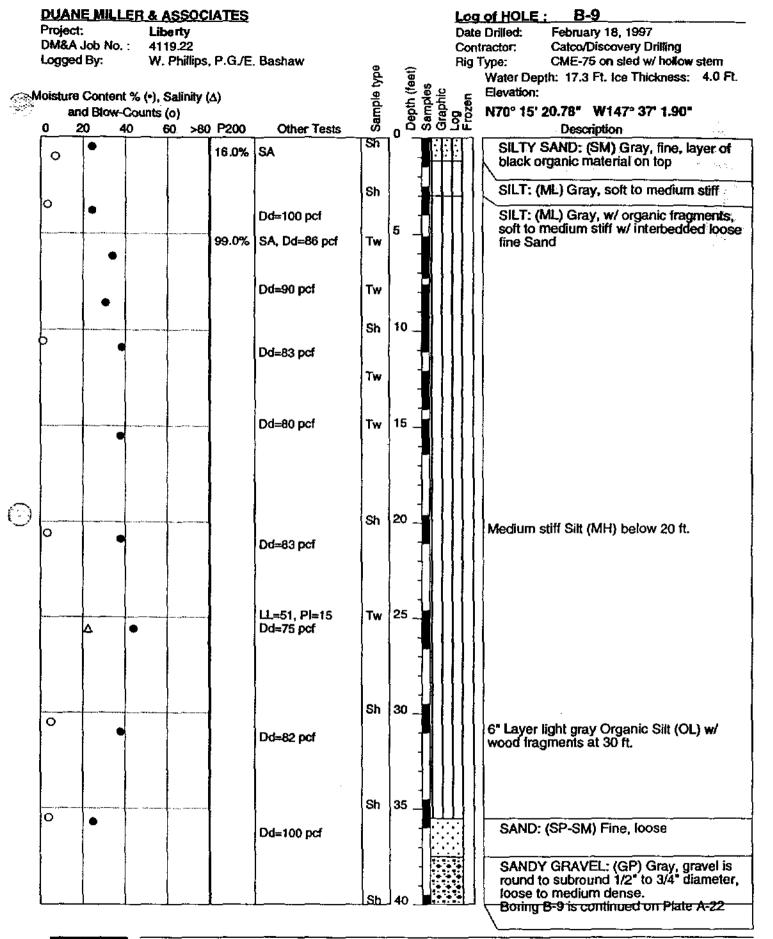
Job No.: 4119.22 Date: June 1997 Liberty Development Beaufort Sea, Alaska





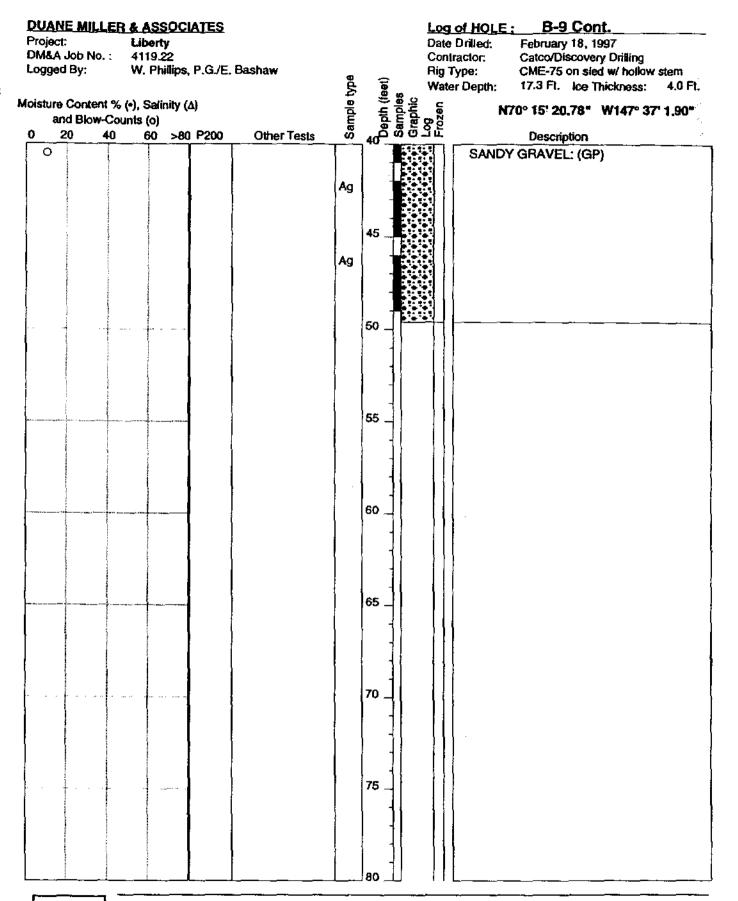
Duane Miller & Associates Arctic & Geotechnical Engineering Job No.: 4119.22

Job No.: 4119.22 Date: June 1997 Liberty Development Beaufort Sea, Alaska



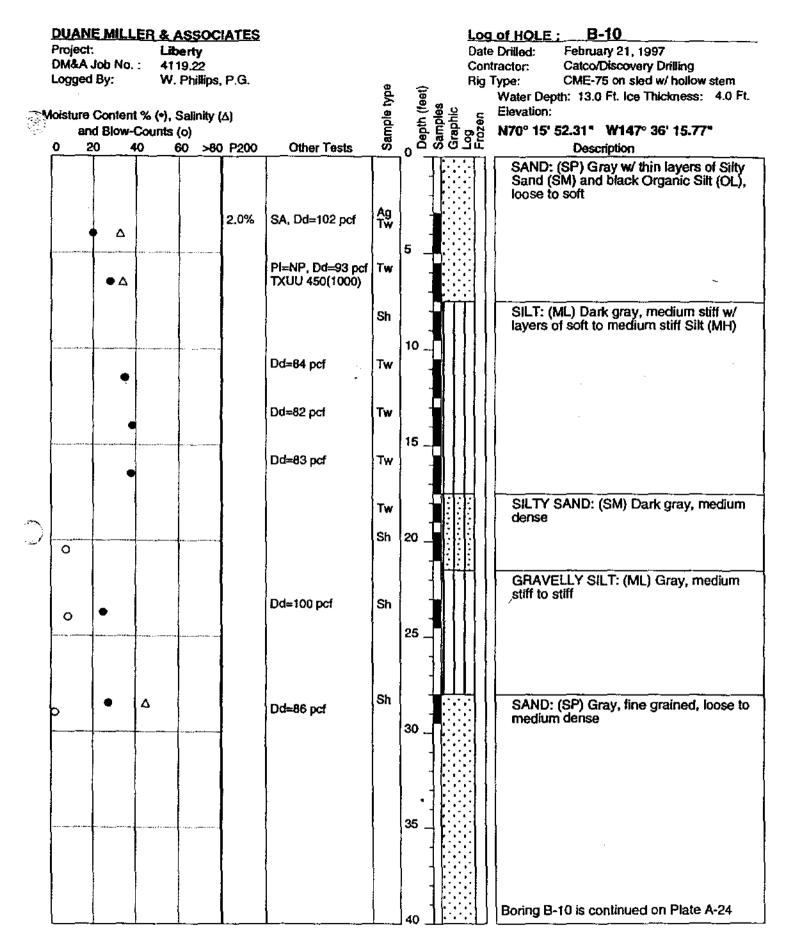


Job No.: 4119.22 Date: June 1997 LOG of BORING B-9 Liberty Development Beaufort Sea, Alaska



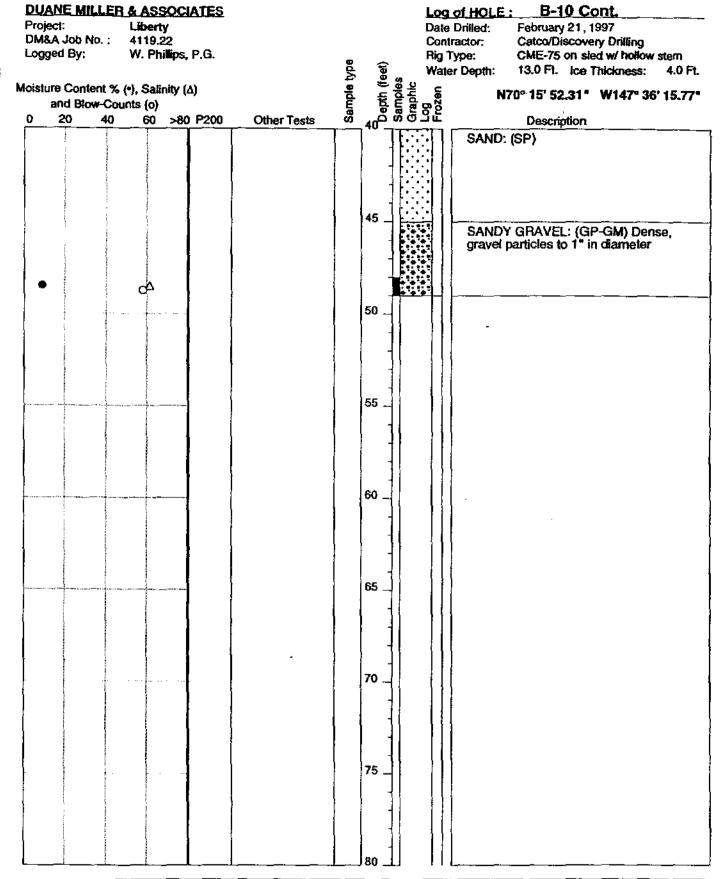


Job No.: 4119.22 Date: June 1997 LOG of BORING B-9 cont. Liberty Development Beaufort Sea, Alaska



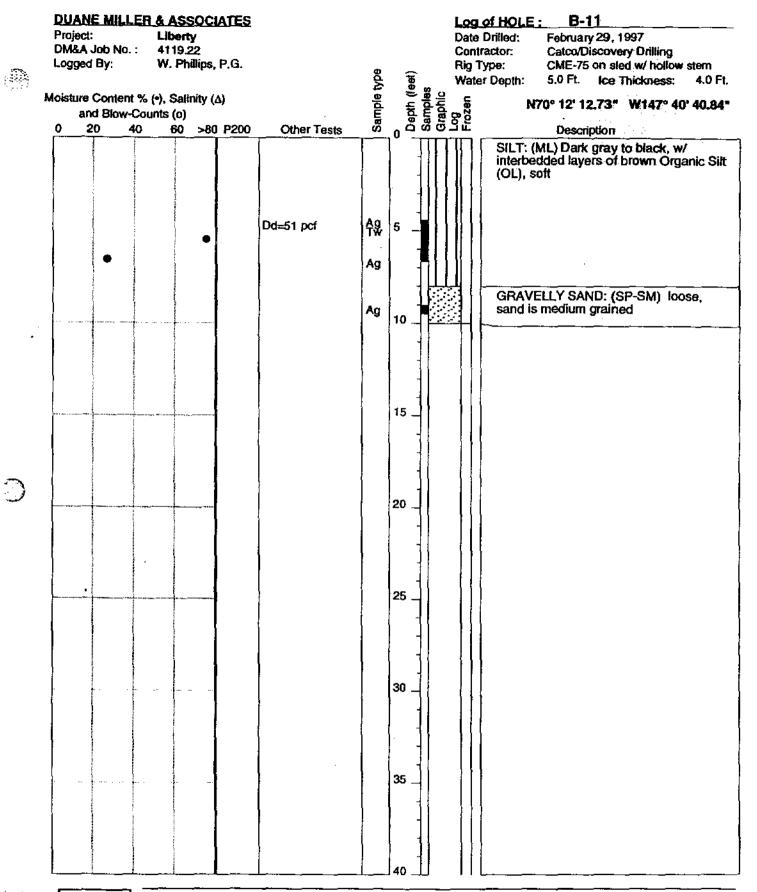


Job No.: 4119.22 Date: June 1997





Job No.: 4119.22 Date: June 1997 LOG of BOBING 8-10 cont. Liberty Development Beaufort Sea, Alaska





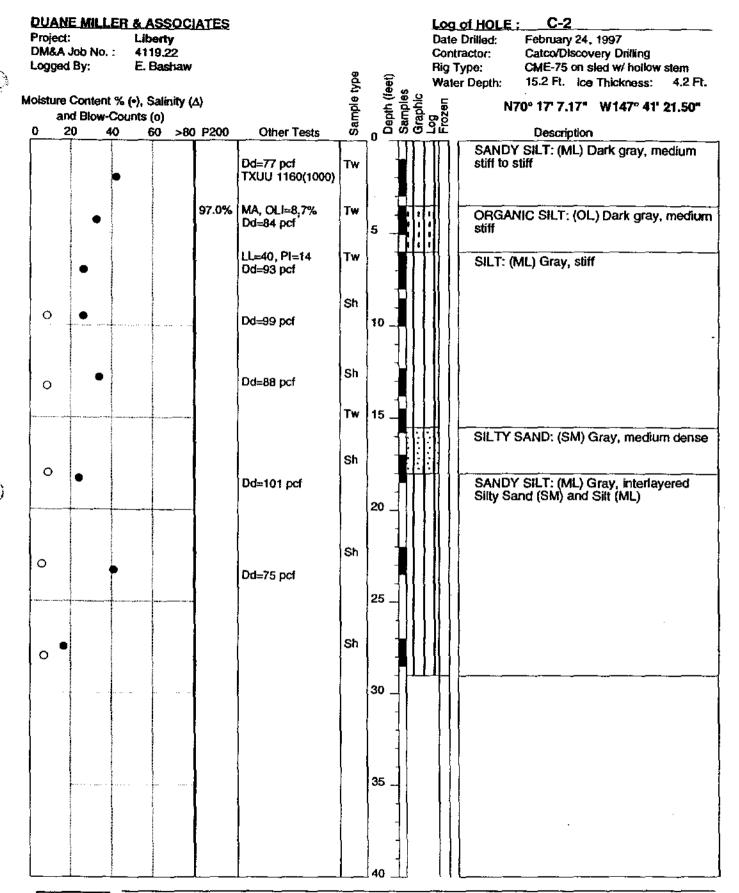
Job No.: 4119.22 Date: June 1997 LOG of BORING B-11 Liberty Development Beaufort Sea, Alaska

DUANE MILLER & ASSOCIATES Log of HOLE: C-1 Liberty Date Drilled: February 25, 1997 DM&A Job No.: 4119,22 Contractor: Catco/Discovery Drilling Logged By: E. Bashaw Rig Type: CME-75 on sled w/ hollow stem Depth (feet) Samples Graphic Log Frozen & 5 Sample type 14.8 Ft. Ice Thickness: Water Depth: 4.3 Ft. Moisture Content % (*), Salinity (A) N70° 16' 24.27" W147° 37' 59.83" and Blow-Counts (o) Other Tests 20 40 >80 P200 Description SAND: (SP) Gray-brown w/ trace gravel. loose Tw SILT: (ML) Dark gray w/ some Clayey Silt (CL-ML), medium stiff to stiff, 6" layer of Sand (SP) at 9' Dd=86 pcf Tw TXUU 1110(1000) Dd=91 pcf Τw TXUU 1010(1000) Dd=86 pcf Tw 10 Dd=84 pcf Tw 15 Sh Dd=86 pcf 0 Trace to some sand and trace organic Sh material in layers below 17' O Dd=109 pcf 20 Sh O Dd=101 pcf 25 Sh 0 30 35



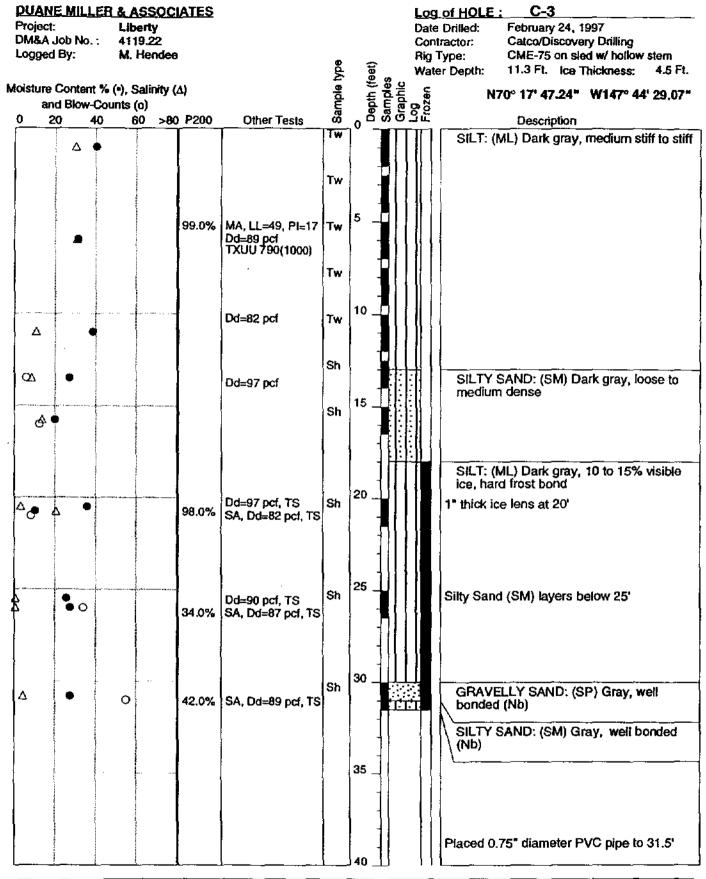
Duane Miller & Associates Arctic & Geotechnical Engineering Job No.: 4119.22

Job No.: 4119.22 Date: June 1997 LOG of BORING C-1 Liberty Development Beaufort Sea, Alaska





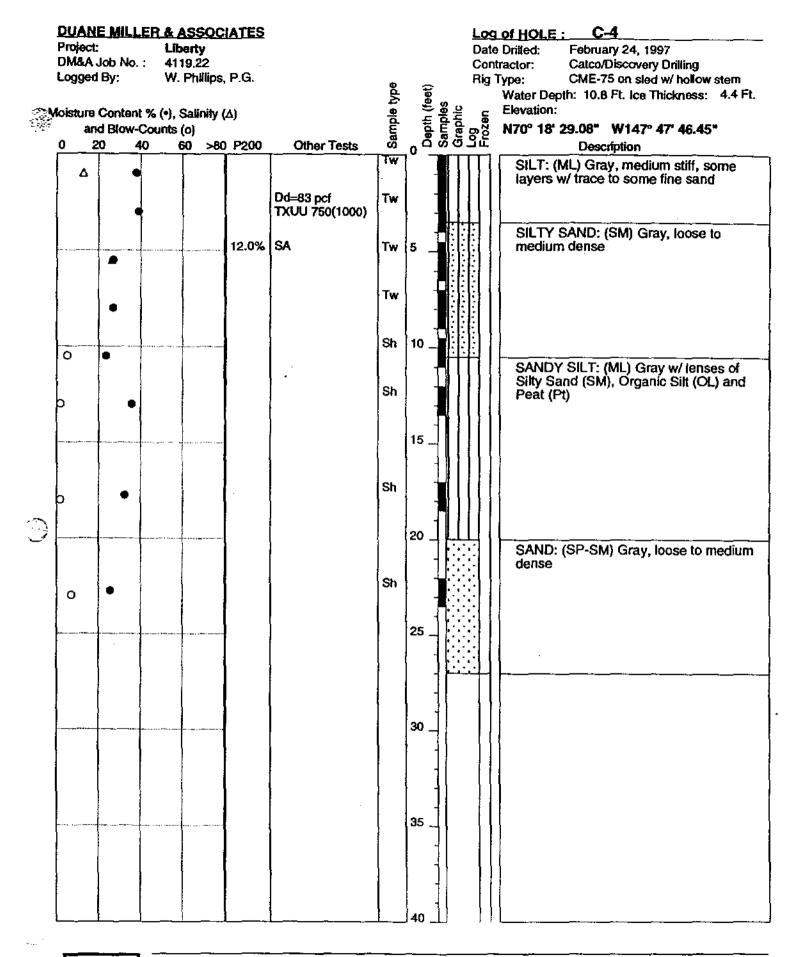
Job No.: 4119.22 Date: June 1997 LOG of BORING C-2 Liberty Development Beaufort Sea, Alaska





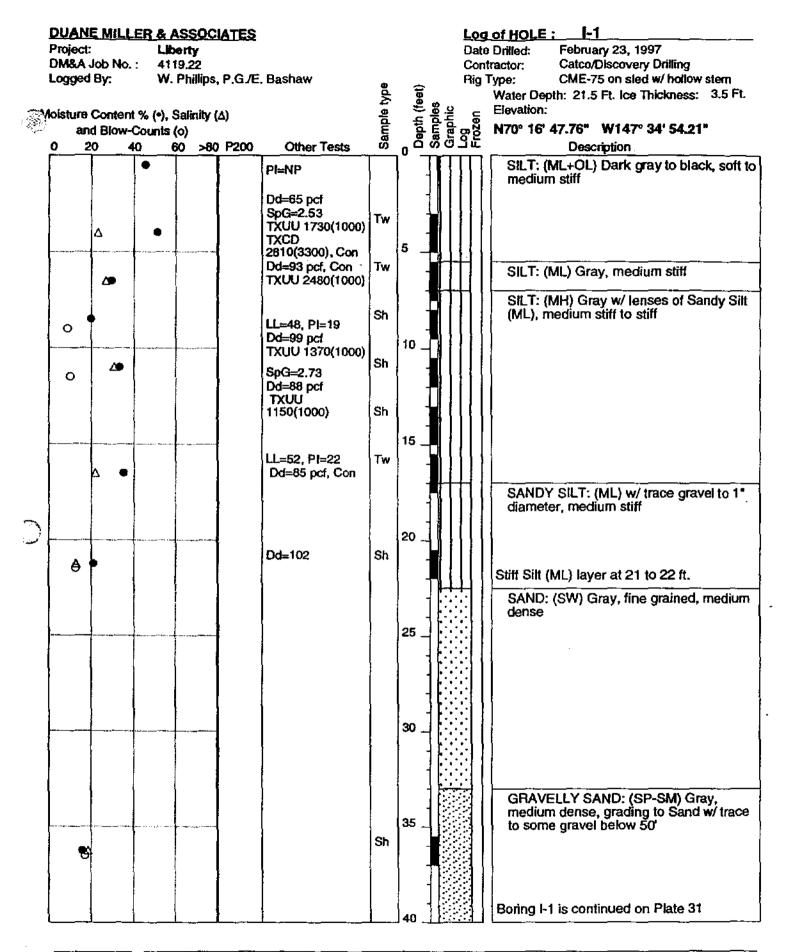
Duane Miller & Associates Arctic & Geotechnical Engineering Job No.: 4119.22

Job No.: 4119.22 Date: June 1997 LOG of BORING C-3 Liberty Development Beaufort Sea, Alaska



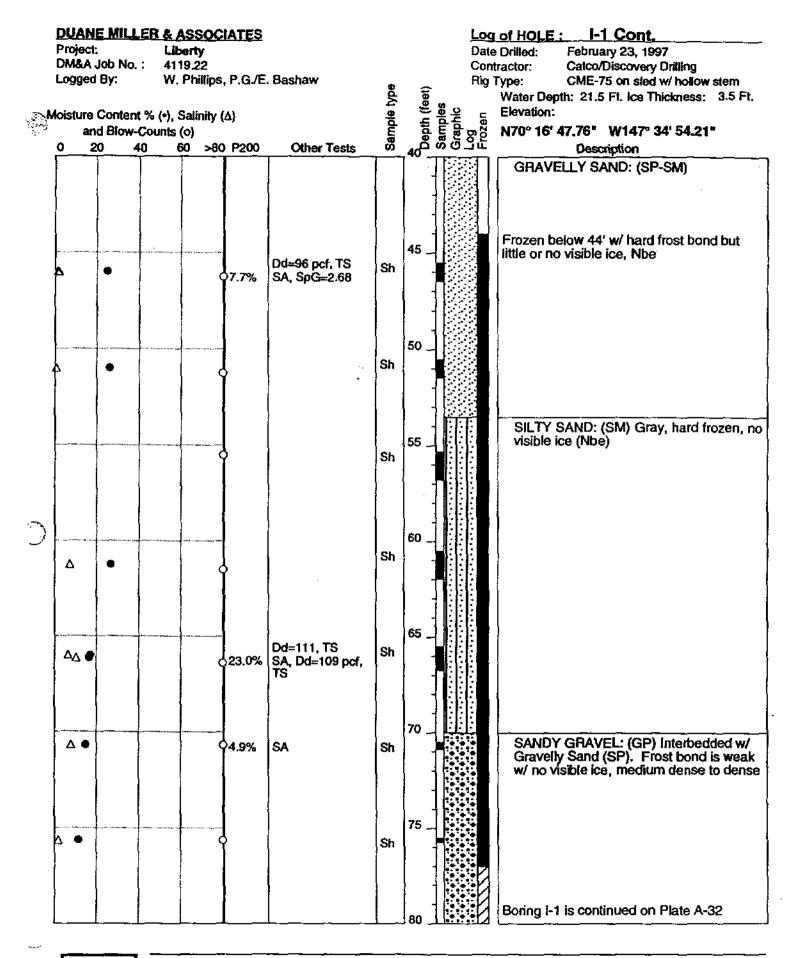


Job No.: 4119.22 Date: September 1997 Liberty Development Beautort Sea, Alaska



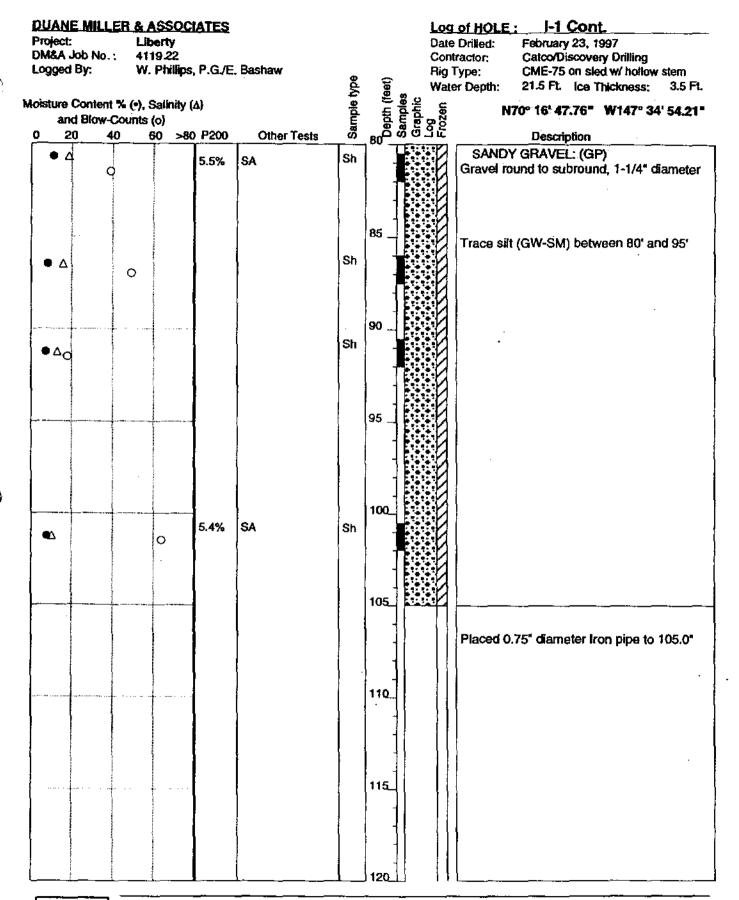


Job No.: 4119.22 Date: September 1997 LOG of ROBING I-1 Liberty Development Beautort Sea, Alaska





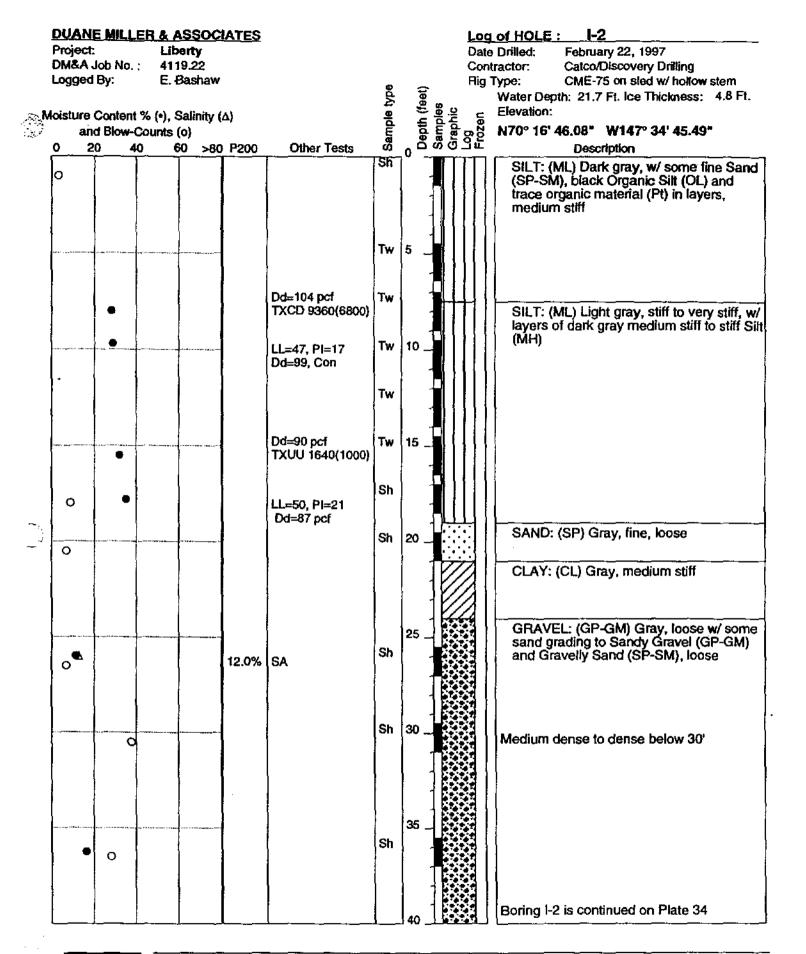
Job No.: 4119.22 Date: September 1997 LOG of BORING I-1 Liberty Development Beaulort Sea, Alaska





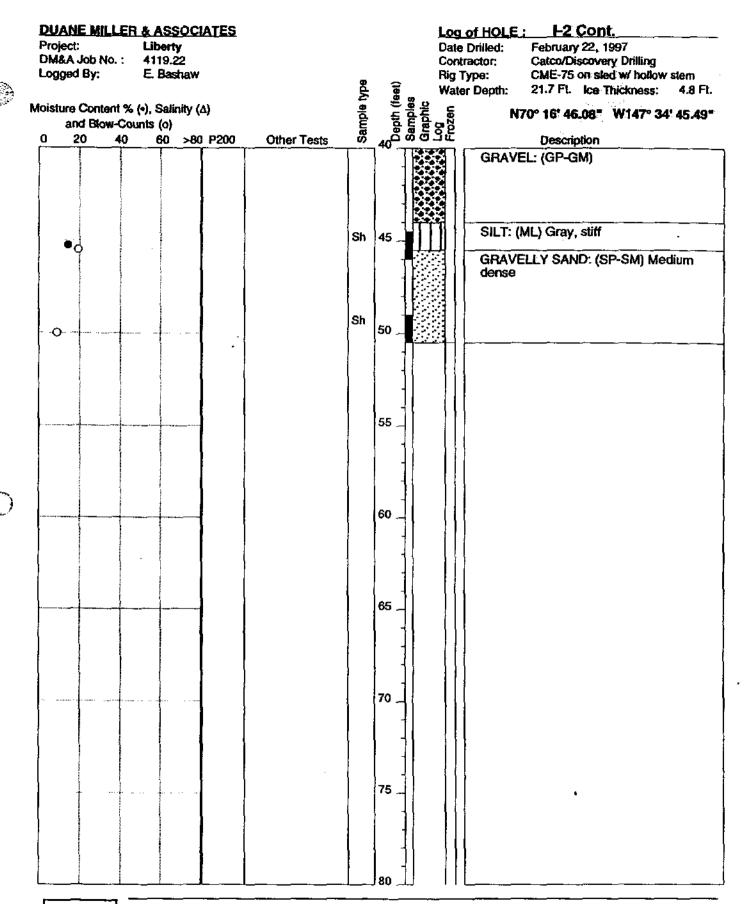
Duane Miller & Associates Arctic & Geotechnical Engineering Job No.: 4119.22

Job No.: 4119.22 Date: June 1997 LOG of BORING 1-1 cont.
Liberty Development
Beaufort Sea, Alaska



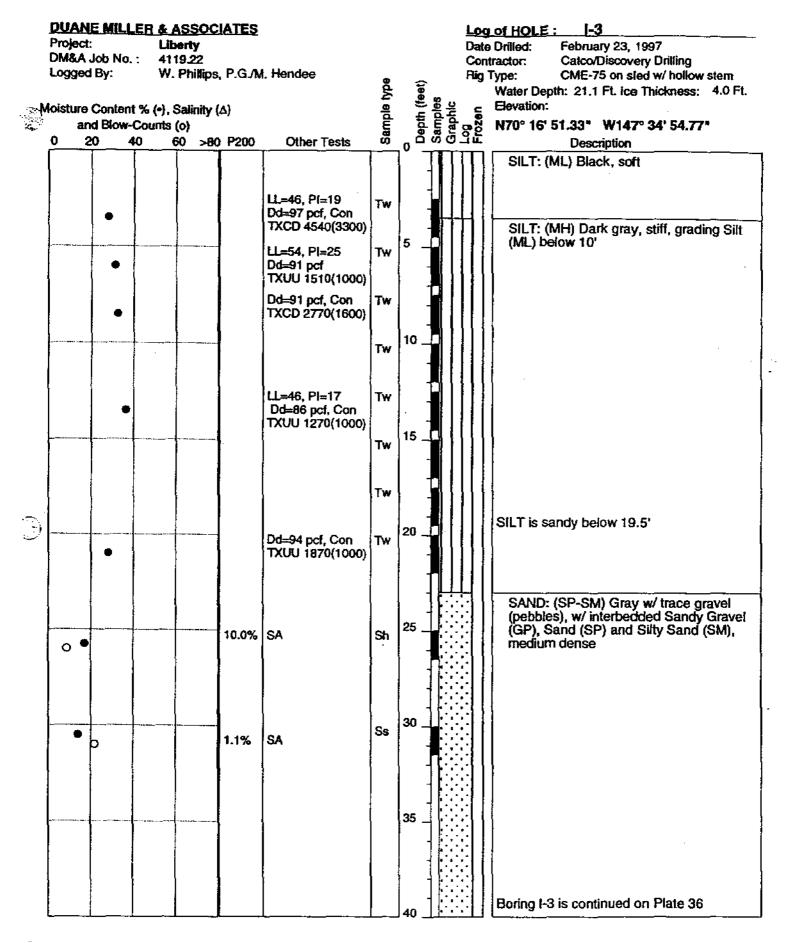


Job No.: 4119.22 Date: June 1997 LIGG of BORING L-2 Liberty Development Beautort Sea, Alaska



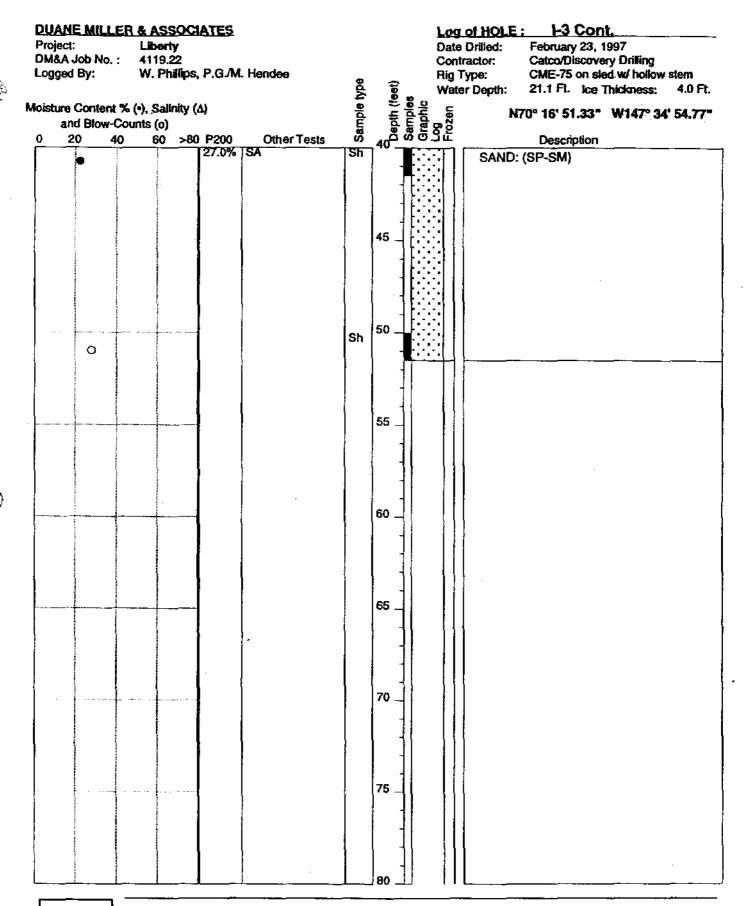


Job No.: 4119.22 Date: June 1997 LOG of BORING I-2 cont. Liberty Development Beaufort Sea, Alaska



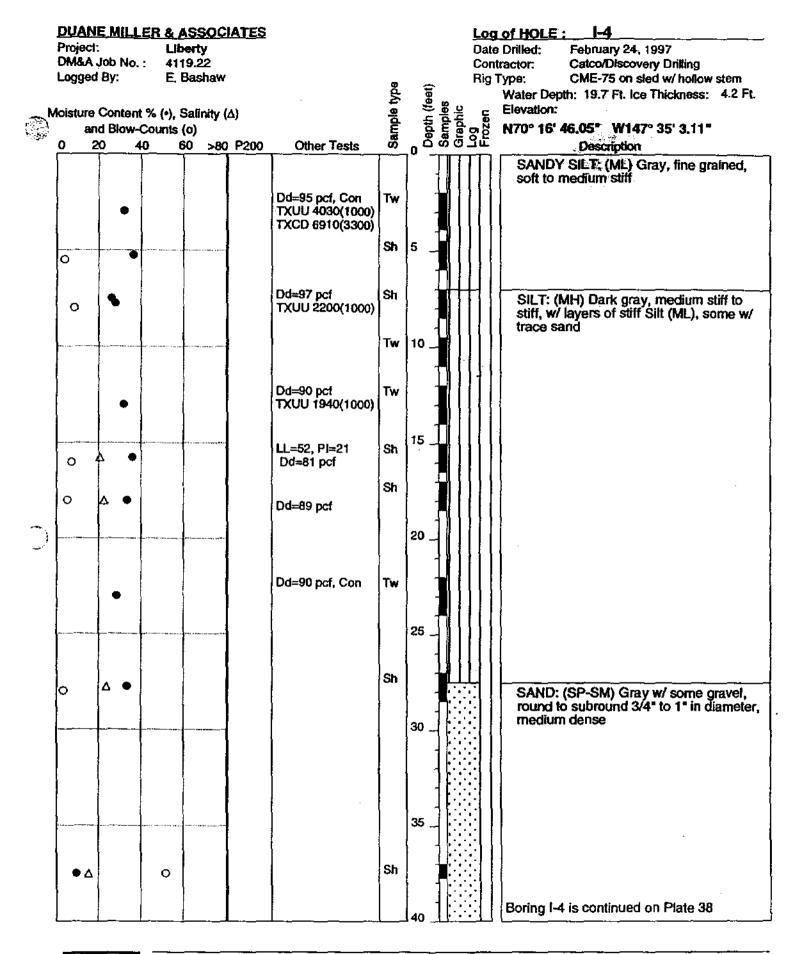


Job No.: 4119.22 Date: June 1997 LOG of BORING 1-3 Liberty Development Beautort Sea, Alaska



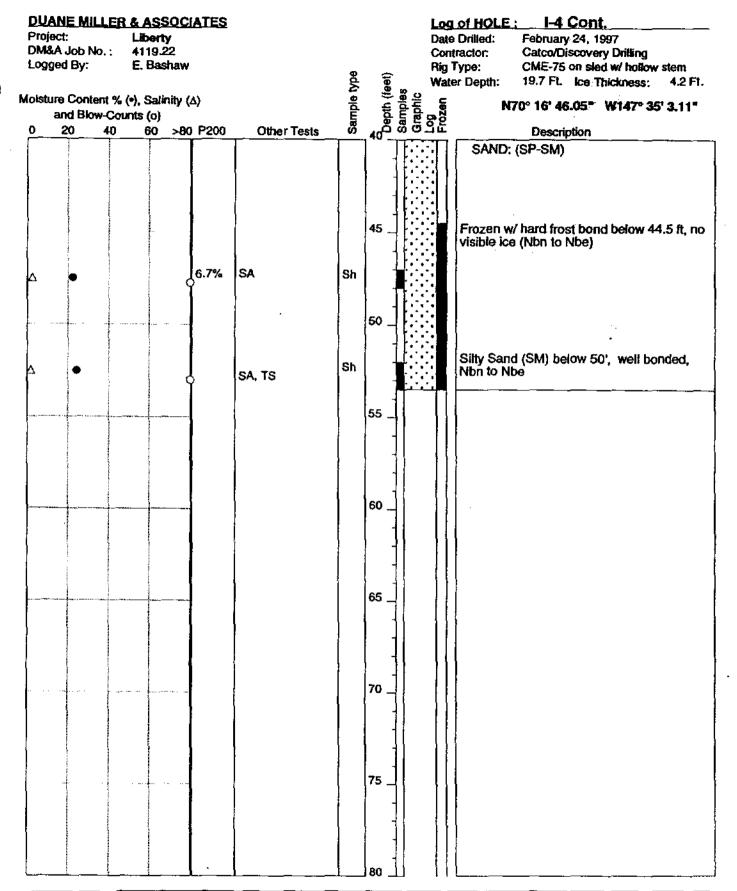


Job No.: 4119.22 Date: June 1997 LOG of BORING L3 cont. Liberty Development Beaufort Sea, Alaska





Job No.: 4119,22 Date: June 1997 Liberty Development Beautort Sea, Alaska





Job No.: 4119.22 Date: June 1997 LOG of BORING 1-4 cont. Liberty Development Beaufort Sea, Alaska

_	MAJOR DIVI	SIONS	SYM	BOL	TYPICAL NAMES				
m /0.		Clean gravels with	GW		Well graded gravels, sandy gravel				
sieve, >0.07 ≀	GRAVELS More than half of the coarse fraction is	little or no fines	GP		Poorty graded gravels, sandy gravel				
918 00	larger than #4 sieve size, > 4.75 mm.	Gravels with more than 12% fines	GM		Silty gravels, silt sand gravel mixtures				
į		aidii 12% iiiles	GC		Clayey gravels, clay sand gravel mixtures				
an 50% larger than #200 a		Clean sands with little or no	sw		Well graded sand, gravelly sand				
! !	SANDS More than half of the coarse fraction	fines	SP		Poorty graded sands, gravelly sand				
mm More than	is smaller than #4 sieve size	Sands with more	SM		Silty sand, silt gravel sand mixtures				
	 	than 12% fines	sc		Clayey sand, clay gravel sand mixtures				
	Plasticity Chart		ML		Inorganic silt and very fine sand, rock flour				
0		SILTS and CLAYS Liquid limit less than 50	CL		Inorganic clay, gravelly and sandy clay, silty clay				
8 00 S	хөрц сн		٥L		Organic silts and clay of low plasticity				
than #	CH MH	CII TO and OI AVO	мн		Inorganic silt				
50%finer than #200 sleve, 0.07	ML 50	SILTS and CLAYS Liquid limit greater than 50	СН		Inorganic clay, fat clay				
2	Liquid Limit		ОН		Organic silt and clay of high plasticity				
	HIGHLY ORGANIC	SOILS	Pt		Peat and other highly organic soil				

KEY TO SAMPLE TYPE

Ag = Auger grab
Ab = Auger bulk
Sh = 2.5" ID split barrel
w/340 lb. manual
hammer
Sha= 2.5" ID split barrel
w/340 lb. automatic
hammer
Sxa = 1.4" ID split barrel
w/340 lb automatic hammer
Tw = Shelby tube
Wc = Rotary wash cut-

KEY TO TEST DATA

DATA

Dd = Dry Density (pcf)
Deg = Degredation
LL = Liquid Limit
P! = Plasticity Index
NP = Non Plastic
SpG = Specific Gravity
SA = Sieve Analysis
MA = Sieve and
Hydrometer Analysis
OLI = Organic Loss on
Ignition (%)
TS = Thaw Strain
TC = Thaw Consolidation
TCf = Thaw Consolidation
TCf = Thaw Consolidation
(field)
TXUU = Unconsolidated
Undrained Triaxial (psf)
TXCD = Consolidated
Drained Triaxial (psf)

XXX (ΥΥΥ) XXX=(σ₁-σ₃)/2 ΥΥΥ=σ₃

UNIFIED SOIL CLASSIFICATION SYSTEM

GROUP	ICE VISIBILITY		DESCRIPTION						
	Coornacted inc. not	Poorly bonded or	Nf						
N	Segregated ice not visible by eye	Well bonded	No excess ice		Nbn				
		Nei bonded	Excess microscopic ice	Nb	Nbe				
	Segregated ice is	Individual ice crys	Vx						
V	visible by eye and	Ice coatings on p	articles		Vc				
	is one inch or less in thickness	Random or irregu	ilarly oriented ice	Vr					
	,	Stratified or distin	ctly oriented ice	Vs					
ICE	ice greater than one inch in thickness	Ice with soil inclu	sions	ICE + soil type					
ICE	inch in thickness	ice without soil in	ICE						

ICE CLASSIFICATION SYSTEM



Duane Miller & Associates
Arctic & Geotechnical Engineering

Job No.: 4119.22 Date: June 1997 SOIL and ICE CLASSIFICATION and KEY TO DATA Liberty Development Beaufort Sea, Alaska

Plate



APPENDIX B

PRIMARY LAB TESTING

Summary of Samples (by Boring No.)

Plates B-1 through B-11

Atterberg LimitS

Plate B-12

Sieve Data

Plates B-13 through B-21

Soil Resistivity

Plate B-22



		Soil Type		Sampling					Sand	Fines							Other
Boring	Depth	(USCS)	Frz?	blows/ft	Content	Density	Salinity	>#4	#4-#200	<#200	LL	PI	PL	Content	Strain	Test 1/2 deviator Sigma 3	Tests
A-1	0.0	Pt	Yes	56												4 4 8 8 8 8	
A-1	0.3'	OL	Yes							ļ	!						
A-1	0.5'	ML	Yes		26.9%												
A-1	5.2'	ML	Yes	54	131.9%												
A-1	6.5'	ML	Yes	48									!				
A-1	11.0	SP	Yes	180	16.3%			32%	60%	7.6%							SA
A-1	16.01	GP	Yes	170	12.6%								i				
A-1	26.0'	GP	Yes	220	11.3%					7.7%							
A-2	0.01	ML	Yes	grab	28.9%												
A-2	6.01	GP	Yes	130	8.6%		49 ppt			3.5%						.	
A-2	11.0	SP	Yes	144	14.2%												
A-2	21.0	SP	Yes	255	7.2%		56 ppt			3.9%		NΡ				* * * * * * * * * * * * * * * * * * *	
A-2	31.0	GP	Yes	300	9.1%		47 ppt										
A-3	0.0'	SP	No	shelby						į			l				•
A-3	5.01	GP	No	17	7.0%		65 ppt	61%	36%	3.2%							SA
A-3	10.0	SP	No	7	10.7%												
A-3	15.0	SP	No	4													
A-3	20.0'	GW	Nο	12	8.5%		İ	62%	36%	1.9%							SA
A-3	25,0'	GP	No	16	10.3%												
A-3	30.0	GP	No	17	9.0%					į							
A-4	0.0'	SP	No	3													
A-4	1.5'	SM	No		23.6%			•		14.3%							
A-4	3.5'	SP	No	8	17.7%	112 pcf		5%	93%	2.2%				·			SA
A-4	6.01	GP	No	8	6.7%			76%	23%	0.7%							SA
A-4	9.5	GP	No	4	6.4%												
A-4	10.5	GP	No	8													
A-4	14.8'	GP	No	10	11.3%												<u></u>

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SUMMARY OF SAMPLES
Liberty Development
Beaufort Sea, Alaska

Plate

B-1



	<u> </u>	Soil Type		Sampling	Moisture	Dry		Gravel	Sand	Fines	Attert	erg Li	mits	Organic	Thaw	Sh	ear Stre	ngth Tests	Other
Boring				blows/ft	Content	Density	Salinity	>#4	#4-#200	<#200		ΡĬ				Test	1/2 devi	ator Sigma 3	Tests
A-4	16.5	GP	No	12															
A-4	20.7	GP	No	7															
A-4	22.3'	GP	No	7															
A-4	30.8'	GP	No	14															
A-5	0.0'	SP-SM	No	4															
A-5	2.7'	SP-SM	No	8	22.4%	107 pcf													
A-5	3.2	ML	Νo	3	35.9%														
A-5	4.21	SP	No		21.9%			0%	98%	1.9%									SA
A-5	6.5'	SP	No	2	24.9%	102 pcf													
A-5	7.5'	ML	No	shelby	52.8%	67 pcf													
A-5	10.5'	GP-GM	No	10	13.2%														
A-5	13.3	GP	No	6						i									
A-5	15.8'	SM	Nο	6	14.4%					13.0%									SA
A-5	25.8	GP	No	64															į
A-5	26.7	ĠP	No	14						į									
A-6	0,0	ML	No	shelby															
A-6	2.8'	ML	No	grab .						į					ļ				
A-6	3.0'	ML+Pt	No	shelby	73.4%	55 pcf	24 ppt				42	6	36			טעאד	90 ps	f 1000 psi	
A-6	5.5'	ML+Pt	No	shelby	47.9%	71 pcf	i							4.0%		TXUU	410 ps	sf 1000 psi	
A-6	7.5'	SM	No	shelby	36.5%	84 pcf	38 ppt	0%	68%	32.2%		NP							SA
A-6	10.0	SM	No	shelby	33.8%	84 pcf													
A-6	12.0'	SM	No	shelby															
A-6	14.0'	SM	No	3	21.3%														1
A-6	18.0	GP	No	13	9.4%					1									
A-6	28.0'	GP	Νo	13	8.1%					İ				•					
A-7	2.01	ML	No	4	26.5%					į				İ					
A-7	3.0'	ML	Νo	shelby	57.2%	65 pcf		0%	9%	91.0%						TXUU	420 p	sf 1000 pst	SA

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SUMMARY OF SAMPLES
Liberty Development
Beaufort Sea, Alaska

		Soli Type		Sampling					Sand	Fines	Atterberg l					ar Strength		Other
				blows/ft	Content	Density	Salinity	>#4	#4-#200	<#200	LL PI	PL	Content	Strain	Test 1	/2 deviator	Sigma 3	Tests
A-7	7.5'	ML	N٥	shelby	Ī													
A-7	9,5'	SM	No	shelby	52.3%	67 pcf												
A-7	14.0'	GP	No	13	8.0%			65%	34%	1.4%								SA
A-7	23.0	GP	No	17	9.9%					į								
A-7	28.0	GP	No	19	7.4%													
A-8	0.0'	ML	No	shelby	50.0%	70 pcf					NP				TXUU	380 psf	1000 psf	
A-8	2.5'	ML	No	shelby	46.2%	72 pcf	19 ppt								TXUU	590 psf	1000 psf	
8-A	5.5'	ML	No	shelby	40.4%	78 pcf	30 ppt	0%	20%	80.0%					,			SA
A-8	8.5'	ML	No	shelby	50.0%	70 pcf									TXUU	540 psf	1000 psf	
8-A	15.0'	SM	No	shelby														
А-в	21.0	SM	No	4	27.6%			0%	78%	22.0%								SA
A-8	25.5'	SM	No	shelby	26.9%	94 pcf	İ											
A-8	26.01	SM	No	6	33.0%													
A-8	36.0	SP-SM	No	4														,
A-9	0.0'	ML.	No	shelby	41.2%	68 pcf				,								
A-9	2.0'	OL	No	shelby	94.8%	46 pcf									TXUU	250 psf	1000 psf	
A-9	4.0'	OL	No	shelby	99.0%	48 pcf										·		
A-9	6.0'	ML	No	shelby	50.2%	68 pcf		0%	13%	87.0%					TXUU	850 psf	1000 psf	МА
A-9	8.01	ML.	Νo	shelby		ŕ												
A-9	10,5'	ML+Pt	No	6	49.1%													
A-9	12.0'	ML.	No	shelby	25.8%	87 pcf												SA
A-9	17.0'	SM	No	shelby		107 pcf												
A-9	22.0'	SM	No	13	13.7%	p		12%	50%	38.0%								SA
A-9	32.0'	SP-SM	No	30	11.6%			,3										

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ML

ML

ML

No

No

No

shelby

shelby

32.3% 83 pcf

50.1% 72 pcf

32.5%

86 pcf

0%

2%

SUMMARY OF SAMPLES
Liberty Development
Beaufort Sea, Alaska

98.0%

MA

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A-10 2.5'

5.5'

7.5

A-10



	·· · · · · · · ·	Soll Type		Sampling					Sand	Fines				Organic		Shear Strength Tests	Other
Boring	Depth	(USCS)	Frz?	blows/ft	Content	Density	Salinity	>#4	#4-#200	<#200	LL	Pi	PL	Content	Strain	Test 1/2 deviator Sigma 3	Tests
A-10	10.0	ML	No	shelby	41.3%	77 pcf										1	1
A-10	12.5	ML	No	shelby	59.6%	64 pcf											
A-10	17.5	ML	No	5	28.6%								į			<u> </u>	**
A-10	22.51	ML	No	3	26.5%												
A-10	23.0	SP	No		22.4%								į			1 1 1 1	
A-10	27.5	SP	No	21	17.0%												***
A-10	32.5'	SP	No	33													•
B-1	0.0	Pt	Yes	34	174.9%												
8-1	1.0'	OL	Yes		214.4%									35.1%			ļ
B-1	2.5'	OL	Yes	33	238.4%												
B-1	7.5'	SM	Yes	57	49.8%		6 ppt	8%	74%	18.0%							SA
B-1	9.5	SM	Yes	86	52.3%												
B-1	11.0°	SM	Yes	48	27.0%	93 pcf	3 ppt	18%	64%	18.0%					1.0%		SA
B-1	14.5	SP-SM	Yes	54	23.0%	99 pcf	2 ppt	3%	86%	11.0%					1.0%		SA
B-1	16.5'	SP-SM	Yes	130	11.3%		16 ppt										
8-1	21.0'	GP-GM	Yes	grab	10.2%		19 ppt									• • • • • • • • • • • • • • • • • • •	SpG=2.65
B-1	26.0	GP-GM	Yes	grab	3.9%		42 ppt										
B-2	0.51	SP	Yes	32	15.3%	113 pcf				1							
B-2	2.4'	GP-GM	Yes	18	15.8%	114 pcf											
B-2	4.8'	GP-GM	Yes	11	l	113 pcf		49%	43%	7.7%			į				SA, SpG=2.67
B-2	6.5'	GP	Yes	42	13.7%	115 pcf	i									! !	ļ
8-2	10.1	SM	Yes	90	9.0%	104 pcf	56 ppt	i	76%	24.0%					3.8%	**************************************	SA
B-2	16.2	GW-GM		180	10.2%		8 ppt	59%	34%	6.9%				•			SA
B-2	26.21	GP	Yes	600	7.9%		8 ppt									5 d d d d d d d d d d d d d d d d d d d	
B-3	0.0	SM	No	3						Ì							
B-3	3.7'	SM	No	shelby													
8-3	6.2'	SM	No	shelby	38.4%		<u> </u>	4%	56%	40.0%					<u> </u>	<u> </u>	SA

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SUMMARY OF SAMPLES
Liberty Development
Beaufort Sea, Alaska

Plate **B-4**





		Soil Type		Sampling					Sand	Fines				Organic			ear Strength		Other
Boring	Depth	(USCS)	Frz?	blows/ft	Content	Density	Salinity	>#4	#4-#200	<#200	LL	Pl	PL	Content	Strain	Test	1/2 deviator	Sigma 3	Tests
B-3	9.5	ML	No	5	23.4%		55 ppt			Ì						<u> </u>			
B-3	10.0'	SM	No	į	24.2%		34 ppt												Í
B-3	11.0	GP-GM	No	8	9.7%		60 ppt			i									
B-3	22.0'	GP-GM	No	19	6.9%		85 ppt			Ì									
B-4	0.0'	ML	No	shelby	40.0%	83 pcf									,				<u>i</u>
B-4	2.5'	ML	No	shelby	33,8%	88 pcf										TXUU	430 psf	1000 psf	
B-4	5,0'	ML	No	shelby												1			***
B-4	8.0'	SP-SM	No	5	17.0%			4%	89%	7.4%									SA
B-4	10,0'	GР	No	7	9.1%			58%	41%	1.1%						! !			SA
B-4	12.5	GP	No	12															
B-5	2.5'	ML	No	shelby	29.5%	91 pcf	32 ppt									TXUU	1310 psf	1000 psf	; ;
B-5	5.0	ML	No	shelby	34.1%	82 pcf		0%	15%	85.0%						TXUU	1200 psf	1000 psf	MA
B-5	7.5'	SP-SM	No	7						į			į						
8-5	10.0	SP-SM	No	9												ļ			1
8-5	15.0'	GP-GM	No	13															į
B-5	25.0'	SP-SM	No	14						1									
B-6	0.0	SP-SM	No	3															
B-6	2.3'	ML	No	shelby	36.3%	85 pcf	23 ppt	0%	3%	97.0%									MA
B-6	4.8'	ML	No	shelby															
B-6	7.3'	ML	No	shelby	30.4%	92 pcf										TXUU	1010 psf	1000 psf	
B-6	9.6'	SP	No	shelby	30.4%	73 pcf		0%	8%	92.0%				_					SA
B-6	13.0	SP-SM	No	13	17.7%					İ				•					
B-6	16.0'	ML	No	5	24.0%														
B-6	18.0'	SM	No	9	18.7%	117 pcf													
B-6	20,6'	SM	No	11	21.2%	106 pcf										<u>.</u>			#
B-6	22.3	GP	Νφ	11												į			
B-6	24.8'	SP	No	9	11.9%												 		<u> </u>

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SUMMARY OF SAMPLES
Liberty Development
Beaufort Sea, Alaska

B-5





		Soil Type		Sampling					Sand	Fines				Organic			ear Strengtl		Other
Boring	Depth	(USCS)	Frz?	blows/ft	Content	Density	Salinity	>#4	#4-#200	<#200	LL	PI	PL	Content	Strain	Test	1/2 deviator	Sigma 3	Tests
B-6	29.81	SP	No	20															1
B-6	35.5'	SP	No	15	8.8%														
B-7	1.0'	SM	No	10	24.6%			į		37.5%						ļ			
B-7	2.1'	ML	No	shelby	38.0%	80 pcf	31 ppt	0%	8%	92.0%									SA
B-7	5.5'	ML	No	2	33.0%	87 pcf		0%	23%	77.0%									МА
B-7	7.1'	ML	No	shelby	24.3%	99 pcf	35 ppt									TXUU	1910 psf	1000 psf	
B-7	9.6'	SM	No	shelby															1
B-7	13.0'	ML,	No	4	71.7%	55 pcf	!												
B-7	16.0'	ML	No	3	76.2%	52 pcf								_					Ī }
B-7	16.5	Pt	No											·					
B-7	17.7	ML.	No	6	19.2%	112 pcf													
B-7	19.8	ML	No	5	20.9%					67.5%									
B-7	24.6'	SM	No	10						į									
B-7	29.6'	SP	No	11															
B-7	31.0'	GP	No	6	11.9%					3.2%									
B-7	40.0	GP	No	7	7.7%					j									
B-8	0.0'	ML	No	shelby															
B-8	2.51	ML	No	shelby	83.8%	49 pcf		0%	4%	96.0%		NP	i			TXUU	370 psf	1000 psf	MA
B-8	5.0'	ML	No	shelby															
B-8	7.5'	ML	No	shelby	41.0%	75 pcf		0%	39%	61.0%									SA
B-8	12.0'	SP	No	4															
B-8	17.0	GP	No	12	15.0%					ļ									
B-8	25.0'	GP	No	grab	8.8%											ļ			1
B-9	0.5'	SM	No	7	24.1%			1%	83%	16.0%									SA
B-9	3.8'	ML	No	3	24.6%	100 pcf													
B-9	5.1'	ML	No	shelby	33.8%	86 pcf		0%	1%	99.0%									SA
B-9	7.61	ML	No	shelby	30.7%	90 pcf													

SUMMARY OF SAMPLES
Liberty Development
Beaufort Sea, Alaska

Plate



Γ'''		Soil Type		Sampling	Moisture	Dry		Gravel	Sand	Fines	Atter	berg L	imits	Organic	Thaw	Sh	ear Stre	ngth Te	sts	Other
Boring		(USCS)												Content						Tests
B-9	10.0'	ML,	No	1	38.2%	83 pcf														
B-9	12.1'	ML	No	shelby										İ						
B-9	14.6'	ML	No	shelby	37.6%	80 pcf														
B-9	20.1	ML	No	3	37.8%	83 pcf														
B-9	24.6	MH	No	shelby	44.4%	75 pcf	23 ppt				51	15	36							<u> </u>
B-9	30,5'	OL	No	5	37.6%	82 pcf								1		<u> </u>				1 1 1 1 1 1
B-9	35.5	SM	No	4	24.8%	100 pcf								ļ						
B-9	39,5'	GP	No	10			j	ı												
B-9	42.0'	GP	No	grab																7 4 4 4 4 8
B-9	46.0	GP	No	grab																<u> </u>
B-10	3.0'	SP	No	shelby	19.7%	102 pcf		0%	98%	2.0%										SA.
B-10	5.5'	SM	No	shelby	28.1%	93 pcf	34 ppt					NP				TXUU	450 p	sf 10	00 psf	
B-10	8.0'	ML	No							į										
B-10	10.5'	ML	No	shelby	35.5%	84 pcf														
B-10	13.0'	ML	No	shelby	38.5%	82 pcf										į				
B-10	15.5	ML	No	shelby	37.9%	83 pcf	į	,												į
B-10	18.0	SM	No	shelby																* * * * * * * * * * * * * * * * * * *
B-10	19.5'	SM	No	7	•		1									<u>.</u> Į				İ
B-10	23.0'	ML	No	8	24.8%	-														
B-10	28.5'	SP	No	2	27.4%	86 pcf	45 ppt													
B-10	48.0'	SP	No	58	8.7%		61 ppt													
B-11	4.5'	ML	No	shelby	75.7%	51 pcf														
B-11	6.5'	OL	No	grab	27.1%									1		1				-
B-11	9.0'	SP-SM	No	grab	40.004											ļ				
B-1A	9.0'	SM	Yes	grab	19.0%									1						
B-1A	11.0'	SM	Yes	98	13.2%	0E	3	100/	600/	19 00/				ļ	25 50					SA
B-1A	14.0	SM	Yes	100	20.7%	85 pcf	3 ppt	15%	68%	13.0%				1	25.5%	<u>1</u>				JOA

SUMMARY OF SAMPLES
Liberty Development
Beaufort Sea, Alaska



		Soil Type		Sampling				Gravel		Fines		berg L		Organic			ear Strength		Other
Boring	Depth	(USCS)	Frz?	blows/ft	Content	Density	Sallnity	>#4	#4-#200	<#200	<u>LL</u>	Pi	PL.	Content	Strain	Test	1/2 deviator	Sigma 3	Tests
8-1A	16.0	SP	Yes	116	13.6%								;		,				1 1 2
B-1A	18.5'	SP-SM	Yes	252	8.2%														
B-1A	21.0	GP-GM	Yes	180	12.6%														h
B-1A	23.0'	GP-GM	Yes	grab															
B-1A	26.0	GP-GM	Yes	200	10.6%														1 1 4 4 4 4 4 4
B-1A	29.5'	GW-GM	Yes	200	14.4%			57%	34%	8.6%			į						SA
C-1	2.0'	SP	No	shelby	35.5%		21 ppt								į				
C-1	4.5'	ML	No	shelby	46.4%	86 pcf										TXUU	1110 psf	1000 psf	
C-1	7.0'	ML	No	shelby	29.1%	91 pcf										TXUU	1010 psf	1000 psf	! !
C-1	9.5'	ML	No	shelby	33.1%	86 pcf													
C-1	12.0'	ML	No	shelby	34.1%	84 pcf							:						
C-1	15.3'	ML	No	6	35.6%	86 pcf													
C-1	18.81	ML	No	6	19.6%	109 pcf							:						
C-1	22.8'	ML	No	8	23.8%	101 pcf	24 ppt												
C-1	27.5	ML	No	9	28.5%		22 ppt												
C-2	1.0'	ML	No	shelby	42.2%	77 pcf				ļ						TXUU	1160 psf	1000 psf	
C-2	3.51	OL	No	shelby	32.4%	84 pcf		0%	3%	97.0%				8.7%					МА
C-2	6.0'	CL-ML	No	shelby	26.2%	93 pcf					40	14	26						! !
C-2	9.5'	CL-ML	No	8	26.2%	99 pcf													L
C-2	12.8	CL-ML	No	8	33.8%	88 pcf													
C-2	14.5'	CL-ML	No	shelby						į			;						
C-2	18.0	ML	No	9	24.5%	101 pcf													
C-2	22.8'	ML	No	6	41.2%	75 pcf				į			;			į			
C-2	27.0	SM	No	7	16.4%														
C-3	0.0	ML	No	shelby	40.4%	79 pcf	30 ppt						;			ΤΧυυ	440 psf	1000 psf	
C-3	2,5'	ML	No	shelby			,												Į
C-3	5.0	ML	No	shelby	31.6%	89 pcf	31 ppt	0%	1%	99.0%	49	17	32			TXUU	790 psf	1000 psf	MA

SUMMARY OF SAMPLES
Liberty Development
Beaufort Sea, Alaska

B-8





		Soll Type		Sampling					Sand	Fines		erg L					ear Strengtl		Other
Boring	Depth	(USCS)	Frz?	blows/ft	Content	Density	Salinity	>#4	#4-#200	<#200	LL	Pl	PL	Content	Strain	Test	1/2 deviator	Sigma 3	Tests
C-3	7.5'	ML	Νo	shelby															i
C-3	10.0	ML.	No	shelby	38.6%	82 pcf	11 ppt												1
C-3	13.5	SM	No	6	27.1%	97 pcf	8 ppt												
C-3	15.0	SM	No	12	19.6%		14 ppt												-
C-3	20.0	ML	Yes		38.6%	77 pcf	21 ppt								13.1%				:
C-3	20.5	ML	Yes	8	35.9%	82 pcf	3 ppt	0%	2%	98.0%					15.3%	! !			SA
C-3	25.5 ¹	SM	Yes		25.7%	90 pcf	0 ppt								6.0%				i
C-3	26.0'	SM	Yes	34	27.7%	87 pcf	0 ppt	0%	66%	34.0%					7.5%	}			SA
C-3	30.8	SM	Yes	55	27.7%	89 pcf	5 ppt	0%	58%	42.0%					6.7%				SA
C-4	0.0	ML	No	shelby	37.6%		13 ppt												
C-4	2.0'	ML	No	shelby	39.0%	83 pcf	i			į						TXUU	750 psf	1000 psf	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
C-4	4.5'	SP-SM	No	shelby	28.6%		27 ppt	0%	88%	12.0%									SA
C-4	7.01	SM	No	shelby	26.9%		ļ	:		į									
C-4	10.5	ML		5	23.7%											į			
C-4	13.0	ML	No	2	35.7%														
C-4	17.0	SM	No	2	32.5%														
C-4	22.0'	SM	No	7	25.6%														'
-1	0.5'	ML+OL	No	shelby	45.5%							NP							***
-1	3.5	ML+OL	No	shelby	50.9%	65 pcf	23 ppt									TXUU	1730 psf	1000 psf	-
J-1	4.0'	ML+OL	No	shelby	50.9%	65 pcf	23 ppt									TXCD	2810 psf	3300 psf	Consol, SpG=2.53
1-1	5.5'	ML	No	shelby	29.6%	93 pcf	27 ppt									TXUU	2480 psf	1000 psf	Consol
J-1	8.5'	ML	No	9	20.0%	99 pcf					48	19	29			TXUU	1370 psf	1000 psf	***
F1	11.0	MH	No	10	33.5%	88 pcf	31 ppt									TXUU	1150 psf	1000 psf	SpG=2.73
I-1	13.5'	МН	No			85 pcf													
1-1	15.5'	MH	No	shelby	35.0%	85 pcf	22 ppt			!	52	22	3						Consol
J-1	20.5'	SM	Νo	13	21.4%	102 pcf	13 ppt												į
1-1	35.5	SP-SM	No	17	16.2%		18 ppt									<u> </u>			

SUMMARY OF SAMPLES
Liberty Development
Beaufort Sea, Alaska

B-9

		Soil Type		Sampling					Sand	Fines		berg L	lmits	Organic	Thaw	Sh	ear Stren	gth Tests	Other
Boring	Depth	(USCS)	Frz?	blows/ft	Content	Density	Salinity	>#4	#4-#200	<#200	LL_	PI	PL_	Content	Strain	Test	1/2 deviat	or Sigma 3	Tests
l-1	45.5'	SP-SM	Yes	110	25.1%	96 pcf	2 ppt	į						į	9.3%				<u> </u>
] I-1	46.0'	SP-SM	Yes				1 ppt	8%	84%	7.7%									SA
l-1	50.5'	SP-SM	Yes	84	26.3%		1 ppt									i 			· ·
l-1	55.5'	SM	Yes	300] }					
-1	60.5'	SM	Yes	84	26.6%		7 ppt									į			
1-1	65.5'	SM	Yes	160	16.5%	111 pcf	10 ppt							ļ ,	5.9%				
-1	66.0	SM	Yes	i	17.3%	109 pcf	6 ppt	3%	74%	23.0%					6.7%				SA
l-1	70.5	SP	Yes	267	14.5%		8 ppt	45%	50%	4.9%									SA
1-1	75.5'	SP	Yes	400	11.7%	•	2 ppt									! ! !			
-1	80.7	GP-GM	?	39	10.6%		18 ppt	51%	44%	5.5%									SA
l - 1	86.5	GP-GM	?	49	8.1%		16 ppt							}		1			
I-1	90.5	GP-GM	?	18	6.9%		13 ppt												
1-1	100.5	GP-GM	7	64	7.5%		10 ppt	53%	42%	5.4%			:	•					SA
1-2	0.0	ML	No	3															
1-2	4.5'	OL	No	shelby															•
1-2	7,0'	SM	No	shelby	28.2%	104 pcf										TXCD	9360 ps	f 6800 psf	
1-2	9.7	ML	No	shelby	28.6%	99 pcf					47	17	30						Consol
1-2	12.0'	ML	No	shelby															
i-2	14.5'	MH	No	shelby	32.1%	90 pcf										TXUU	1640 ps	f 1000 psf	į
1-2	17.8'	MH	No	9	35.1%	87 pcf					50	21	29	<u>.</u>					
I-2	19,5'	SP	No	7			İ							į		ļ			
1-2	26.0'	GP-GM	No	7	11.3%		13 ppt	77%	11%	12.0%									SA
1-2	29.5'	GP-GM	No	38										į					
1-2	35.5'	SP-SM	No	28	16.5%					!				İ					
1-2	44.5'	ML	No	19	14.1%														
I-2	49.0	SP-SM	No	9						j			:						
1-3	2.5'	ML	No	shelby	28.3%	97 pcf					46	19	27			TXCD	4540 ps	f 3300 psf	Consol

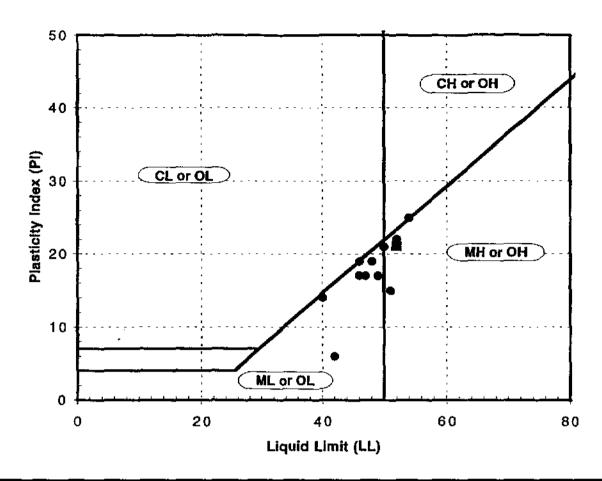
SUMMARY OF SAMPLES
Liberty Development
Beaufort Sea, Alaska



		Soil Type		Sampling				Gravel		Fines				Organic			near Strength		Other
Boring	Depth	(USCS)	Frz?	blows/ft	Content	Density	Salinity	>#4	#4-#200	<#200	LL	PI	PL	Content	Strain	Test	1/2 deviator	Sigma 3	Tests
I-3	5.0'	МН	No	shelby	31.1%	91 pcf		İ			54	25	29			TXUU	1510 psf	1000 psf	
∣ 1•3	7.5'	MH	No	shelby	32.4%	91 pcf										TXCD	2770 psf	1600 psf	Consol
1-3	10.0	MH	No	shelby												İ			4 4 4
1-3	12.5'	ML	No	shelby	36.2%	86 pcf					46	17	29			TXUU	1270 psf	1000 psf	Consol
1-3	15,0'	ML	No	shelby										<u> </u>					•
1-3	17.5'	ML	No	sheiby															
1-3	20.0	ML	No	shelby	28.2%	94 pcf										TXUU	1870 psf	1000 psf	Consol
1-3	25.0'	SP-SM	No	9	17.2%			2%	88%	10.0%									SA
1-3	30.5	SP	No	22	14.3%			21%	78%	1.1%				<u>.</u>		İ			SA
1-3	40.0	SM	No	14	22.2%			4%	69%	27.0%									SA
1-3	50.0'	SP	No	28										Í		Ī			
1-4	2.0'	SM	No	shelby	32.0%	95 pcf										TXUU	4030 psf	1000 psf	Consol
1-4	2.01	SM	No	shelby	32.0%	95 pcf										TXCD	6910 psf	3300 psf	Consol
1-4	4.5'	ML	No	4	36.8%					į						İ			i i
1-4	7.0'	MH	No		28.2%	97 pcf										TXUU	2200 psf	1000 psf	
1-4	7.51	MH	No	8	26.4%	97 pcf				į				t t					
1-4	9.5'	MH	No	shelby															
1-4	12.0'	MH	No	shelby	32.3%	90 pcf				<u> </u>						TXUU	1940 psf	1000 psf	
1-4	15.0	MH	No	7	35.6%	81 pcf	20 ppt				52	21	31						
1-4	18.0	MH	No	5	33.5%	89 pcf	22 ppt												
1-4	22.0'	MH	No	shelby	28.7%	90 pcf													Consol
1-4	27.0'	MH	No	3	33.1%		24 ppt												
1-4	37.0'	SP-SM	No	52	9.7%		15 ppt												
1-4	47.0'	SP-SM	Yes	142	22.5%		3 ppt	5%	88%	6.7%									SA
1-4	52.5	SM	Yes	93	24.1%	95 pcf	2 ppt	0%	87%	13.0%					6.3%				SA

SA = sieve analysis, MA = hydrometer and sieve analysis, SpG = specific gravity, Consol = consolidation test

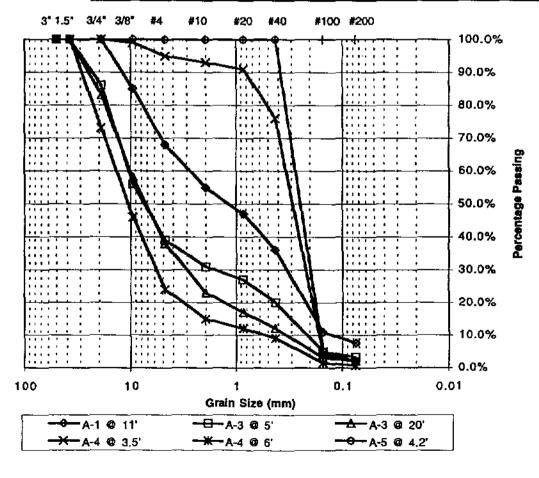
SUMMARY OF SAMPLES
Liberty Development
Beaufort Sea, Alaska



Sample Leastion	Liquid	Plastic	Plasticity	Natural Moisture	USCS
Sample Location	<u>Limit</u>	Limit	Index	Content	
Boring A-2 @ 21.0'			NP	7.2%	SP
Boring A-6 @ 3.0'	42	36	6	66.2%	ML + Pt
Boring A-6 @ 7.5'			ΝP	36.5%	SM
Boring A-8 @ 0.0'			NP	50.0%	ML
Boring B-8 @2.5			NP	83.8%	ML
Boring B-9 @ 24.6'	51	36	15	22.6%	MH
Boring B-10 @ 5.5'			NP	28.1%	SM
Boring C-2 @ 6.0*	40	26	14	26.2%	CL-ML
Boring C-3 @ 5.0'	49	32	17	31.6%	ML
Boring I-1 @ 0.5'			NP	45.5%	ML + OL
Boring !-1 @ 8.5'	48	29	19	20.0%	ML
Boring I-1 @ 15.5'	52	30	22	35.0%	MH
Boring I-2 @ 9.7'	47	30	17	28.6%	ML
Boring I-2 @ 17.8'	50	29	21	35.1%	MH
Boring I-3 @ 2.5'	46	27	19	28.3%	ML
Boring I-3 @ 5.0'	54	29	25	31.1%	MH
Boring I-3 @ 12.5'	46	29	17	36.2%	ML
Boring I-4 @ 15.0'	52	31	21	<u>35.6%</u>	MH_

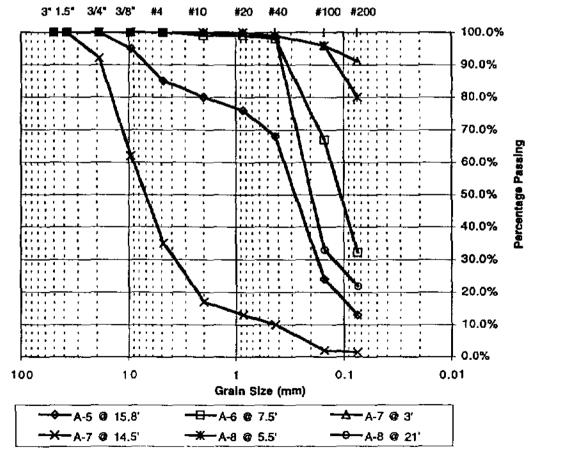
Boring => Depth =>	A-1 11.0 ft.	A-3 5.0 ft.	A-3 20.0 ft.	A-4 3.5 ft.	A-4 6.0 ft.	A-5 4.2 ft.
3" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
1 1/2" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
3/4* =>	100.0%	86.0%	83.0%	100.0%	73.0%	100.0%
3/8" ⇒>	85.0%	56.0%	58.0%	99.0%	46.0%	100.0%
#4 =>	68.0%	39.0%	38.0%	95,0%	24.0%	100.0%
#10 =>	55.0%	31.0%	23.0%	93.0%	15.0%	100.0%
#20 =>	47.0%	27.0%	17.0%	91.0%	12.0%	100.0%
#40 =>	36.0%	20.0%	12.0%	76.0%	9.0%	100.0%
#100 =>	11.0%	5.0%	3.0%	4.0%	1.5%	5.0%
#200 =>	7.6%	3.2%	1.9%	2.2%	0.7%	1.9%
0.02 mm		i				
0.005 mm]		•	,	
0.002 mm		<u> </u>				

Analysis of Data						
D10 size =>	0.122 mm	0.070 mm	0.337 mm	0.164 mm	0.535 mm	0.158 mm
D30 size =>	0.331 mm	1.615 mm	2.995 mm	0.218 mm	5.738 mm	0.197 mm
D50 size =>	1.172 mm	7.438 mm	7.200 mm	0.292 mm	10.527 mm	0.246 mm
D60 size =>	2.789 mm	10.420 mm	10.042 mm	0.337 mm	13.609 mm	0.274 mm
Coeff. of Uniformity, Cu =	22.80	148.85	29.78	2.06	25.41	1.73
Coeff. of Curvature, Cc =	0.32	3.58	2.65	0.87	4,52	0.90
Grave! (+#4) percentage =	32.0%	61.0%	62.0%	5.0%	76.0%	0.0%
AASHTO Gravel (+#10) =	45.0%	69.0%	77.0%	7.0%	85.0%	0.0%
Sand percentage =	60.4%	35.8%	36.1%	92.8%	23.3%	98.1%
Fines percentage =	7.6%	32%	1.9%	2.2%	0.7%	1.9%
Unified Soil Class Symbol =	SP-SM	GP	GW	SP	GP	SP



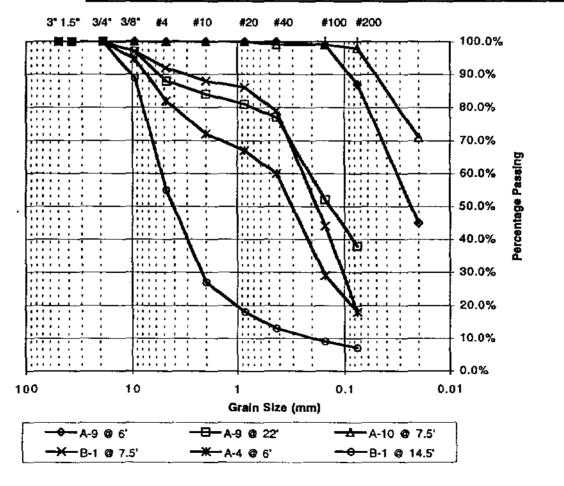
Boring =>	A-5	A-6	A-7	A-7	A-8	A-8
Depth ≈>	15.8 ft.	7.5 ft.	3.0 ft.	14.5 ft.	5.5 ft.	21.0 ft.
3° =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
1 1/2" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
3/4" =>	100.0%	100.0%	100.0%	92.0%	100.0%	100.0%
3/8" =>	95.0%	100.0%	100.0%	62.0%	100.0%	100.0%
#4 =>	85.0%	100.0%	100.0%	35.0%	100.0%	100.0%
#10 =>	80.0%	99.0%	100.0%	17.0%	100.0%	100.0%
#20 =>	76.0%	99.0%	99.0%	13.0%	100.0%	100.0%
#40 =>	68.0%	98.0%	99.0%	10.0%	99.0%	99.0%
#100 =>	24.0%	67.0%	96.0%	2.0%	96.0%	33.0%
#200 =>	13.0%	32.2%	91.0%	1.4%	80.0%	22.0%
0.02 mm						
0.005 mm		l .				
0.002 mm		<u> </u>				

			0.425 mm		T
0.173 mm			3.735 mm		0.124 mm
0.278 mm	0.107 mm		6.981 mm		0.196 mm
0.352 mm	0.130 mm		9.025 mm		0.230 mm
Ï			21.23		İ
			3.64		
15.0%	0.0%	0.0%	65.0%	0.0%	0.0%
20.0%	1.0%	0.0%	83.0%	0.0%	0.0%
72.0%	67.8%	9.0%	33.6%	20.0%	78.0%
13.0%	32.2%	91.0%	1.4%	80.0%	22.0%
SM	SM	ML	GP	ML	SM
	0.278 mm 0.352 mm 15.0% 20.0% 72.0% 13.0%	0.278 mm	0.278 mm	0.173 mm 3.735 mm 0.278 mm 0.107 mm 6.981 mm 0.352 mm 0.130 mm 9.025 mm 21.23 3.64 15.0% 0.0% 0.0% 65.0% 20.0% 1.0% 0.0% 83.0% 72.0% 67.8% 9.0% 33.6% 13.0% 32.2% 91.0% 1.4%	0.173 mm 3.735 mm 0.278 mm 0.107 mm 6.981 mm 0.352 mm 0.130 mm 9.025 mm 21.23 3.64 15.0% 0.0% 0.0% 65.0% 0.0% 20.0% 1.0% 0.0% 83.0% 0.0% 72.0% 67.8% 9.0% 33.6% 20.0% 13.0% 32.2% 91.0% 1.4% 80.0%



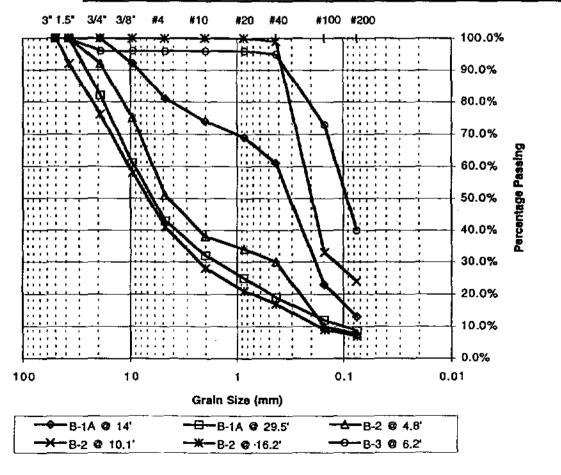
Boring =>	A -9	A-9	A-10	B-1	B-1	B-1
Depth ⇔	6.0 ft.	22.0 ft.	7.5 ft.	7.5 ft.	11.0 ft.	14.5 ft.
3" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
1 1/2" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
3/4" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
3/8* ⇒>	100.0%	97.0%	100.0%	97.0%	94.7%	89.0%
#4 =>·	100.0%	88.0%	100.0%	92.0%	82.0%	55.0%
#10 =>	100.0%	84.0%	100.0%	88.0%	72.0%	27.0%
#20 =>	100.0%	81.0%	100.0%	86.0%	67.0%	18.0%
#40 =>	100.0%	77.0%	99.0%	79.0%	60.0%	13.0%
#100 =>	99.0%	52.0%	99.0%	44.0%	29.0%	9.0%
#200 =>	87.0%	38.0%	98.0%	18.0%	18.0%	7.2%
0.02 mm	45%		71%		, 	
0.005 mm						
0.002 mm	16%		22%			

Analysis of Data _						
D10 size =>		T				0.195 mm
D30 size =>		, ,		0.103 mm	0.155 mm	2.194 mm
D50 size =>		0.136 mm		0.179 mm	0.304 mm	4.070 mm
D60 size =>		0.209 mm		0.241 mm	0.425 mm	5.260 mm
Coeff. of Uniformity, Cu =		1				27.03
Coeff. of Curvature, Cc ≃		1 1		1		4.70
Gravel (+#4) percentage =	0.0%	12.0%	0.0%	8.0%	18.0%	45.0%
AASHTO Gravel (+#10) =	0.0%	16.0%	0.0%	12.0%	28.0%	73.0%
Sand percentage =	13.0%	50.0%	2.0%	74.0%	64.0%	47.8%
Fines percentage ≃	87.0%	38.0%	98.0%	18.0%	18.0%	72%
Unified Soil Class Symbol =	ML	SM	ML	SM	SM	SP-SM



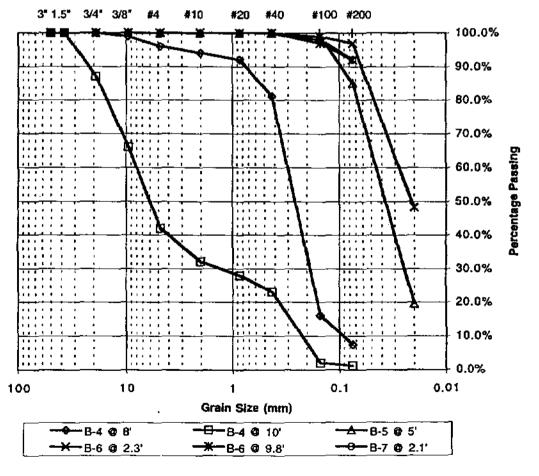
Boring => Depth =>	B-1A 14.0 ft.	B-1A 29.5 ft.	B-2 4.8 ft.	B-2 10.1 ft.	B-2 16.2 ft.	B-3 6.2 ft.
3" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
1 1/2" =>	100.0%	100.0%	100.0%	100.0%	92.0%	100.0%
3/4" =>	100.0%	82.0%	92.0%	100.0%	76.0%	96.0%
3/8° ⇒>	92.0%	61.0%	75.0%	100.0%	58.0%	96.0%
#4 =>	81.0%	43.0%	51.0%	100.0%	41.0%	96.0%
#10 =>	74.0%	32.0%	38.0%	100.0%	28.0%	96.0%
#20 =>	69.0%	25.0%	34.0%	100.0%	21.0%	96.0%
#40 =>	61.0%	19.0%	30.0%	99.0%	17.0%	95.0%
#100 =>	23.0%	12.0%	10.0%	33.0%	9.0%	73.0%
#200 =>	13.0%	8.6%	7.7%	24.0%	6.9%	40.0%
0.02 mm						
0.005 mm	'					
0.002 mm						

Allalysis of Data						
D10 size =>		0.100 mm	0.150 mm		0.171 mm	
D30 size =>	0.182 mm	1.566 mm	0.425 mm	0.119 mm	2.285 mm	
D50 size =>	0.314 mm	6.220 mm	4,444 mm	0.196 mm	6.856 mm	0.093 mm
D60 size =>	0.414 mm	9.141 mm	6.160 mm	0.230 mm	10.261 mm	0.114 mm
Coeff. of Uniformity, Cu =		91.62	41.07		60.05	
Coeff. of Curvature, Cc =		2.69	0.20		2.98	
Gravel (+#4) percentage ≈	19.0%	57.0%	49.0%	0.0%	59.0%	4.0%
AASHTO Gravel (+#10) =	26.0%	68.0%	62.0%	0.0%	72.0%	4.0%
Sand percentage =	68.0%	34.4%	43.3%	76.0%	34.1%	56.0%
Fines percentage =	13.0%	8.6%	7.7%	24.0%	6.9%	40.0%
Unified Soil Class Symbol =	SM	GW-GM	GP-GM	SM	GW-GM	SM



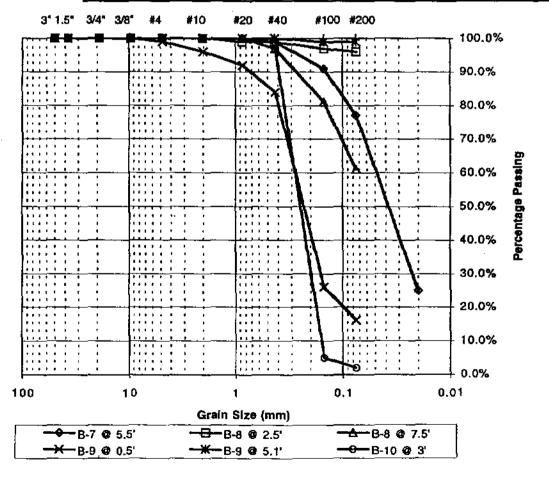
Boring =>	B-4	B-4	B-5	B-6	B-6	B-7
Depth =>	8.0 ft.	10.0 ft.	5.0 ft.	2.3 ft.	9.8 ft.	2.1_ft
3⁼ =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
1 1/2" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
3/4" =>	100.0%	87.0%	100.0%	100.0%	100.0%	100.0%
3/8" =>	99.0%	66.0%	100.0%	100.0%	100.0%	100.0%
#4 =>	96.0%	42.0%	100.0%	100.0%	100.0%	100.0%
#10 =>	94.0%	32.0%	100.0%	100.0%	100.0%	100.0%
#20 =>	92.0%	28.0%	100.0%	100.0%	100.0%	100.0%
#40 =>	81.0%	23.0%	100.0%	100.0%	100.0%	100.0%
#100 =>	16.0%	2.0% i	99.0%	99.0%	97.0%	98.0%
#200 =>	7.4%	1.1%	85.0%	97.0%	92.0%	92.0%
0.02 mm	1		20%	48%		
0.005 mm	·	1	Ì]	
0.002 mm			8%	9%		

Allalysis VI Dala _						
D10 size =>	0.092 mm	0.223 mm	0.003 mm	0.002 mm		
D30 size =>	0.188 mm	1.304 mm	0.025 mm	0.005 mm		
D50 size ⇒>	0.259 mm	5.985 mm	0.038 mm	0.022 mm		
D60 size =>:	0.304 mm	7.989 mm	0.046 mm	0.029 mm		[
Coeff. of Uniformity, Cu =	3.28	35.82	18.40	14.50	•	
Coeff. of Curvature, Cc =	1.26	0.95	5.43	0.43		
Gravel (+#4) percentage =	4.0%	58.0%	0.0%	0.0%	0.0%	0.0%
AASHTO Gravel (+#10) =	6.0%	68.0%	0.0%	0.0%	0.0%	0.0%
Sand percentage =	88.6%	40.9%	15.0%	3.0%	8.0%	8.0%
Fines percentage =	7.4%	1.1%	85.0%	97.0%	92.0%	92.0%
Unified Soil Class Symbol =	SP-SM	GP	ML	ML	MŁ	ML



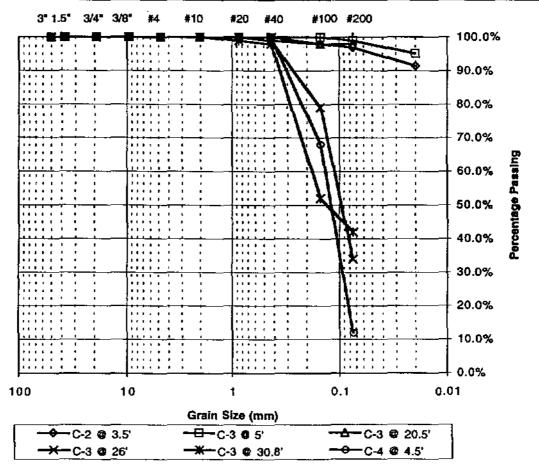
Boring ⇒ Depth ⇒	B-7 5.5 ft.	B-8 2.5 ft.	B-8 7.5 ft.	B-9 0.5 ft.	B-9 5,1 ft.	B-10 3.0 ft.
3* =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
1 1/2" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
3/4" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
3/8" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
#4 =>	100.0%	100.0%	100.0%	99.0%	100.0%	100.0%
#10 =>	100.0%	100.0%	100.0%	96.0%	100.0%	100.0%
#20 =>	100.0%	99.0%	100.0%	92.0%	100.0%	100.0%
#40 ⇒>	99.0%	99.0%	97.0%	84.0%	100.0%	97.0%
#100 ==>	91.0%	97.0%	81.0%	26.0%	99.0%	5.0%
#200 =>	77.0%	96.0%	61.0%	16.0%	99.0%	2.0%
0.02 mm	25%	50%				
0.005 mm	11%					
0.002 mm	8%	14%				

mm
mm
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)mm (
76]
B9
)% j
)%
0%
<u>%</u>
P



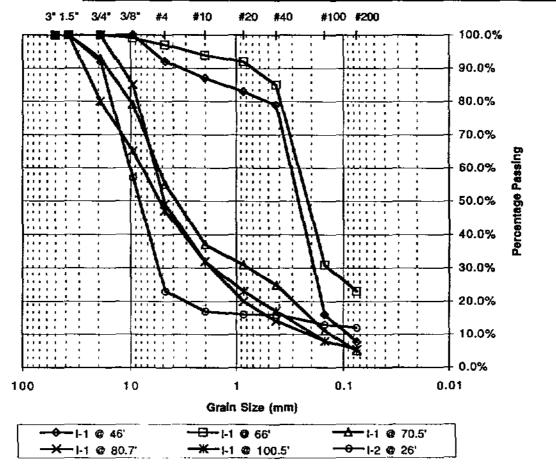
Boring =>	C-2	C-3	C-3	C-3	C-3	C-4
Depth =>	3.5 ft.	5.0 ft.	20.5 ft.	26.0 ft.	30.8 ft.	4.5 ft.
3" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
1 1/2" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
3/4" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
3/8" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
#4 =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
#10 =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
#20 =>	100.0%	100.0%	100.0%	100.0%	99.0%	100.0%
#40 =>	99.0%	100.0%	100.0%	99.0%	98.0%	100.0%
#100 =>	98.0%	100.0%	98.0%	79.0%	52.0%	68.0%
#200 =>	97.0%	99.0%	98.0%	34.0%	42.0%	12.0%
0.02 mm	92%	95%				
0.005 mm						
0.002 mm	45%	38%				

Allarysis Ul Dala _						
D10 size =>		·				
D30 size =>		į				0.094 mm
D50 size =>		•		0.096 mm	0.131 mm	0.120 mm
D60 size =>				0.112 mm	0.180 mm	0.136 mm
Coeff. of Uniformity, Cu =			[·			
Coeff. of Curvature, Cc =						i
Gravel (+#4) percentage =	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%
AASHTO Gravel (+#10) =	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%
Sand percentage =	3.0%	1.0%	2.0%	66.0%	58.0%	88.0%
Fines percentage =	97.0%	99.0%	98.0%	34.0%	42.0%	12.0%
Unified Soil Class Symbol =	OL.	ML.	ML	SM	SM	SP



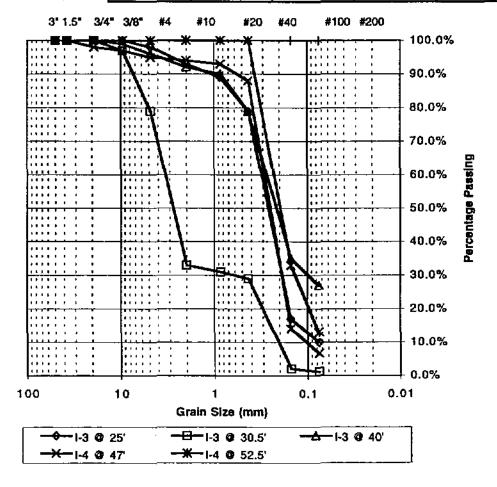
Boring => Depth =>	I-1 46.0 ft.	I-1 66.0 ft.	i-1 70.5 ft.	£-1 80.7 ft.	I-1 100.5 ft.	I-2 26.0 ft.
3* =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
1 1/2" =>	100.0%	100.0%	100.0%	100.0%	100.0%	100.0%
3/4" =>	100.0%	100.0%	93.0%	100.0%	80.0%	92.0%
3/8" =>	100.0%	99.0%	79.0%	85.0%	65.0%	57.0%
#4 =>	92.0%	97.0%	55.0%	49.0%	47.0%	23.0%
#10 =>	87.0%	94.0%	37.0%	32.0%	32.0%	17.0%
#20 =>	83.0%	92.0%	31.0%	20.0%	23.0%	16.0%
#40 =>	79.0%	85.0%	25.0%	14.0%	17.0%	16.0%
#100 =>	16.0%	31.0%	11.0%	8.0%	8.0%	13.0%
#200 =>	7.7%	23.0%	4.9%	5.5%	5.4%	12.0%
0.02 mm						
0.005 mm						
0.002 mm						

Analysis of Data						
D10 size =>	0.091 mm		0.134 mm	0.212 mm	0.189 mm	
D30 size =>	0.189 mm	0.138 mm	0.757 mm	1.734 mm	1.654 mm	5.479 mm
D50 size =>	0.263 mm	0.216 mm	3.735 mm	4.842 mm	5.332 mm	8.237 mm
D60 size =>	0.310 mm	0.262 mm	5.488 mm	5.870 mm	7.836 mm	10.082 mm
Coeff. of Uniformity, Cu =	3.42	ļ	40.99	27.66	41.45	•
Coeff. of Curvature, Cc =	1.27		0.78	2.41	1.85	
Gravel (+#4) percentage =	8.0%	3.0%	45.0%	51.0%	53.0%	77.0%
AASHTO Gravel (+#10) =	13.0%	6.0%	63.0%	68.0%	68.0%	83.0%
Sand percentage =	84.3%	74.0%	50.1%	43.5%	41.6%	11.0%
Fines percentage =	7.7%	23.0%	4.9%	5.5%	5.4%	12.0%
Unified Soil Class Symbol ≈	SP-SM	SM	SP	GW-GM	GW-GM	GP-GM



Boring => Depth =>	I-3 25.0 ft.	1-3 30.5 ft.	I-3 40.0 ft.	I-4 47.0 ft.	I-4 52.5 ft.
3" =>	100.0%	100.0%	100.0%	100.0%	100.0%
1 1/2" =>	100.0%	100.0%	100.0%	100.0%	100.0%
3/4" =>	100.0%	100.0%	100.0%	98.0%	100.0%
3/8" =>	100.0%	97.0%	99.0%	97.0%	100.0%
#4 =>	98.0%	79.0%	96.0%	95.0%	100.0%
#10 =>	93.0%	33.0%	92.0%	94.0%	100.0%
#20 =>	89.0%	31.0%	90.0%	93.0%	100.0%
#40 =>	79.0%	29.0%	79.0%	88.0%	100.0%
#100 =>	17.0%	2.0%	35.0%	14.0%	33.0%
#200 =>	10.0%	1.1%	27.0%	6.7%	13.0%
0.02 mm					
0.005 mm					
0.002 mm					

Allalysis Ul Data _					
D10 size ⇒>	<u>-</u>	0.204 mm		0.103 mm	
D30 size =>	0.187 mm	0.601 mm	0.097 mm	0.188 mm	0.135 mm
D50 size =>	0.261 mm	2.753 mm	0.214 mm	0.249 mm	0.195 mm
D60 size =>	0.309 mm	3.323 mm	0.271 mm	0.287 mm	0.228 mm
Coeff. of Uniformity, Cu =		16.27		2.79	
Coeff. of Curvature, Cc =		0.53		1.20	
Gravel (+#4) percentage =	2.0%	21.0%	4.0%	5.0%	0.0%
AASHTO Gravei (+#10) =	7.0%	67.0%	8.0%	6.0%	0.0%
Sand percentage =	88.0%	77.9%	69.0%	88.3%	87.0%
Fines percentage =	10.0%	1.1%	27.0 <u>%</u>	6.7%	13.0%
Unified Soil Class Symbol =	SP-SM	SP	SM	SP-SM	SM



RISM

Proceeding Tables

BIOT VANGUARD OR ANCHORAGE ALASKA 99507 PH 907-522-1707

Resistivity ASTM G-57

CLIENTIPROJECT: Duane Miller & Associates

TEST ON: Field Samples

source: Liberty Development

SAMPLED FROM: Test Holes

REM PROJECT:

651017

LAB NO: 712

7125

DATE REPORTED:

4/16/97

DATE RECEIVED:

3/12/97

	Test Hole	Depth	ohm-centimeter	% Moisture
	A.C.	7.5		20 5
1/	<u>A-6</u>	<u>7.5</u>	<u>61</u>	<u>38.5</u>
2/	<u>C-4</u>	<u>4.5</u>	<u>140</u>	<u>26.6</u>
3/	B-10	<u>3.0</u>	130	<u>19.7</u>
4/	<u>B-7</u>	<u>2.1</u>	<u>78</u>	<u>38</u>
5/	<u>C-1</u>	<u>2.0</u>	<u>85</u>	<u>35.5</u>
6/	C-1 C-4 A-3	<u>0.0</u>	<u>86</u>	<u>37.6</u>
7/	A-3	<u>5.0</u>	<u>200</u>	<u>10</u>
8/	<u>B-10</u>	<u>5.5</u>	<u>130</u>	<u>21.6</u>
9/	1-1	3.0	<u>61</u>	<u>58.4</u>
10/	<u>F1</u>	<u>5.5</u>	<u>120</u>	<u>35.3</u>
11/	<u>B-5</u>	<u>2.5</u>	<u>78</u>	<u>32</u> 28.2
12/	<u>B-7</u>	<u>7.1</u>	<u>95</u>	<u>28.2</u>

Notes: Tests were conducted with a Geotest (Nilsson) Model 400, 4-Pin Soil Resistance Meter

Tested By

Signed By



Duane Miller & Associates Arctic & Geotechnical Engineering

Job No.: 4119.22 Date: September 1997

APPENDIX C

STRENGTH TESTING

Triaxial Data

PROJECT : LIBERTY

LAB + T97007; 0 = 7.0 psi; 8 = 55.1 pcf

CLIENT : DURNE MILLER

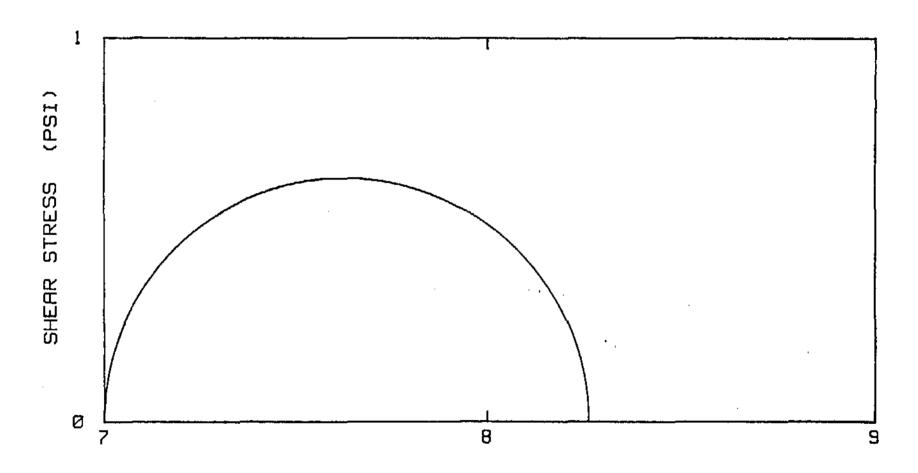
W.O. # A27153

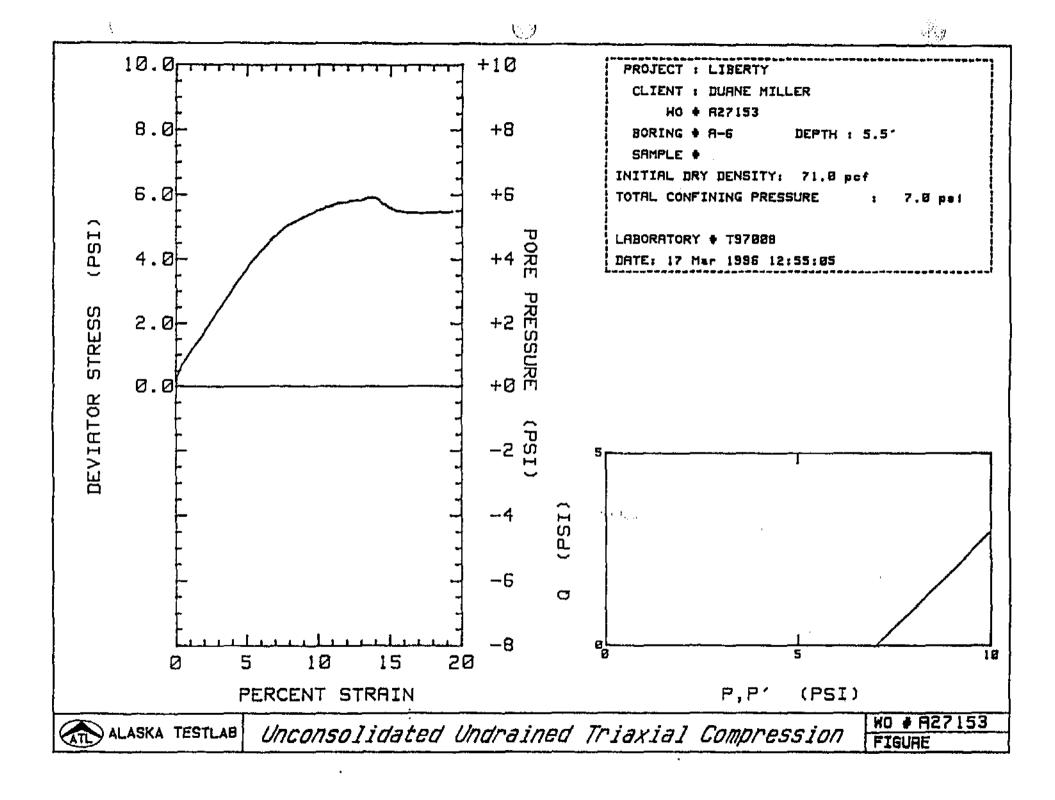
BORING # A-6

SAMPLE +

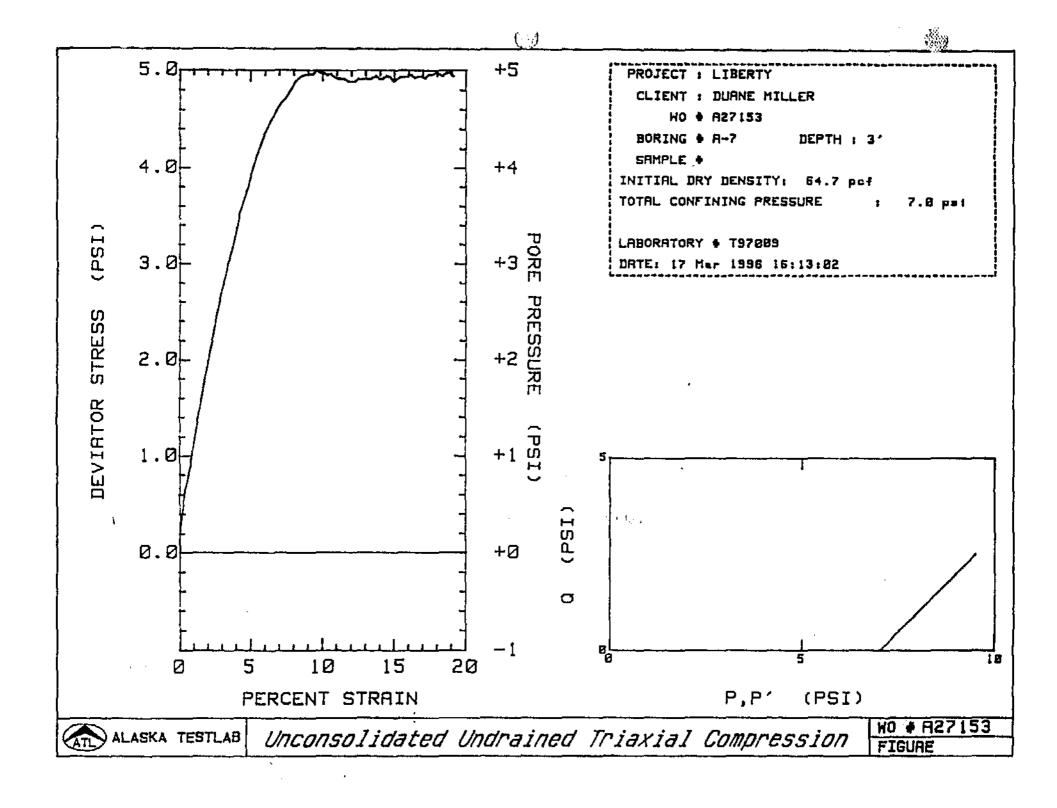
DEPTH: 3'

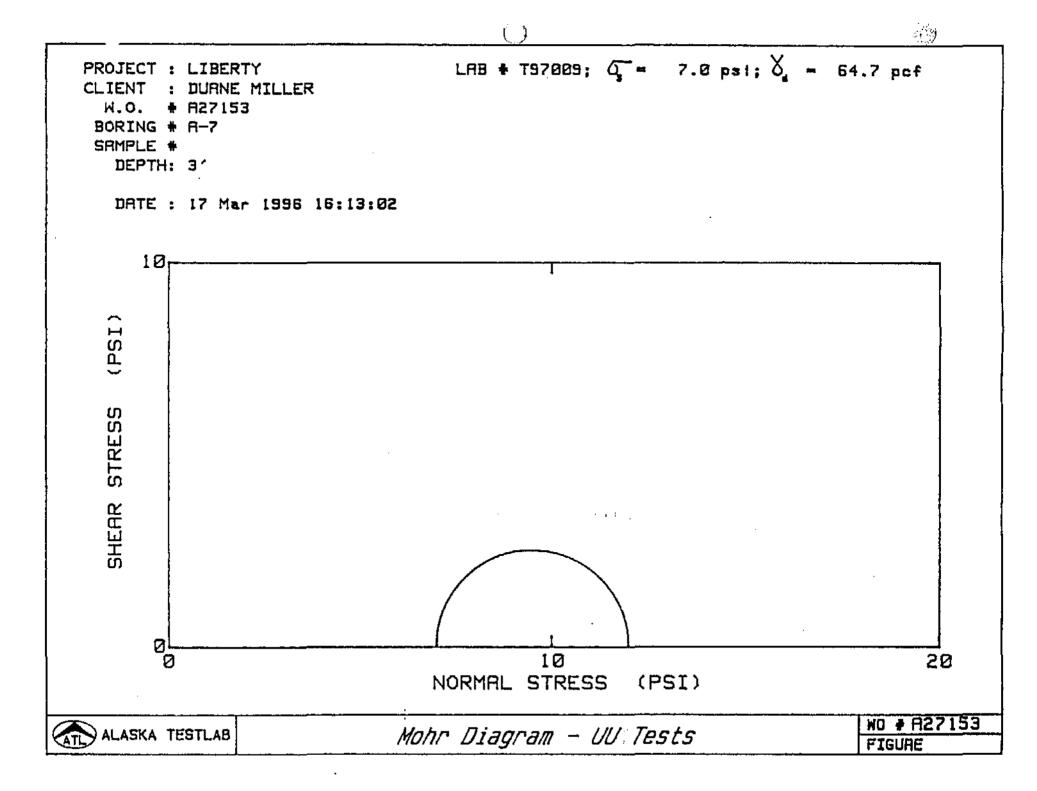
DATE : 17 Mar 1996 07:52:45

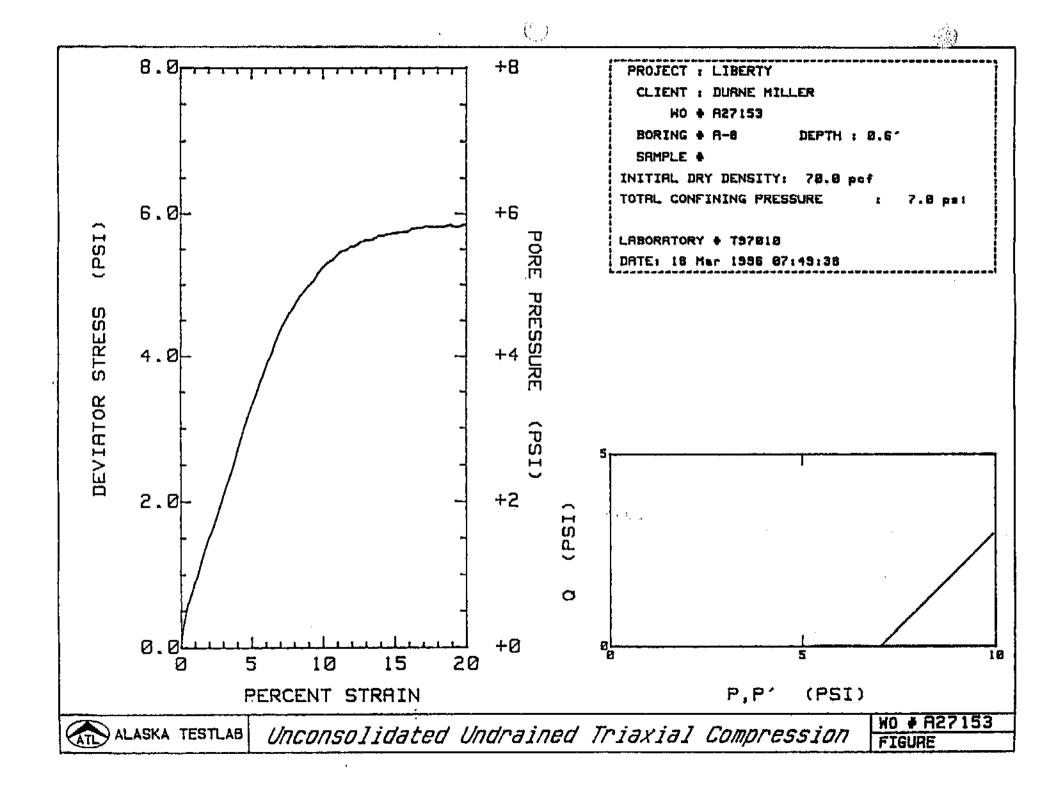




LAB * T97008; $\delta_{s} = 7.0 \text{ psi; } \delta_{s} = 71.0 \text{ pcf}$ PROJECT : LIBERTY CLIENT : DURNE MILLER W.O. # #27153 BORING # A-6 SAMPLE # DEPTH: 5.5' DRTE: 17 Mar 1996 12:55:05 10 (PSI) STRESS SHEAR 10 20 NORMAL STRESS (PSI) WO # A27153 ALASKA TESTLAB Mohr Diagram - UU Tests FIGURE







PROJECT : LIBERTY

LAB + T97010; 0 = 7.0 psi; 0 = 70.0 pcf

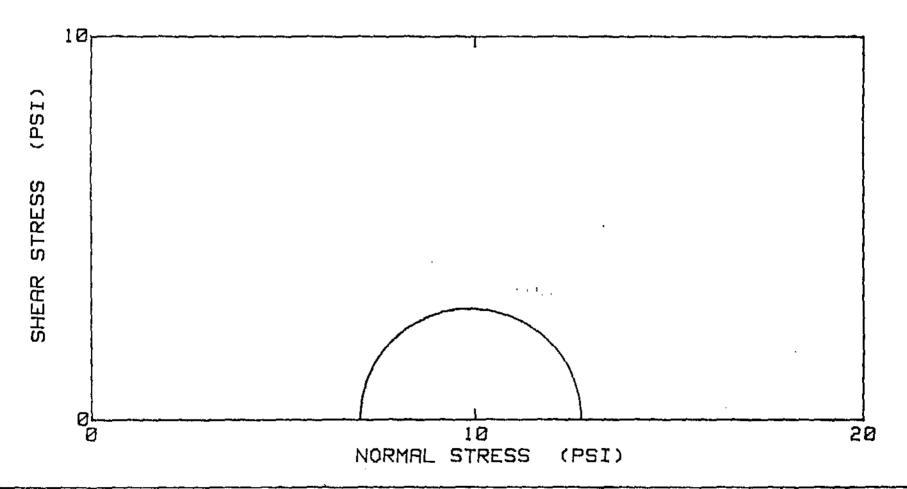
CLIENT : DUANE MILLER

W.O. # A27153 BORING # A-8

SAMPLE #

DEPTH: 0.6'

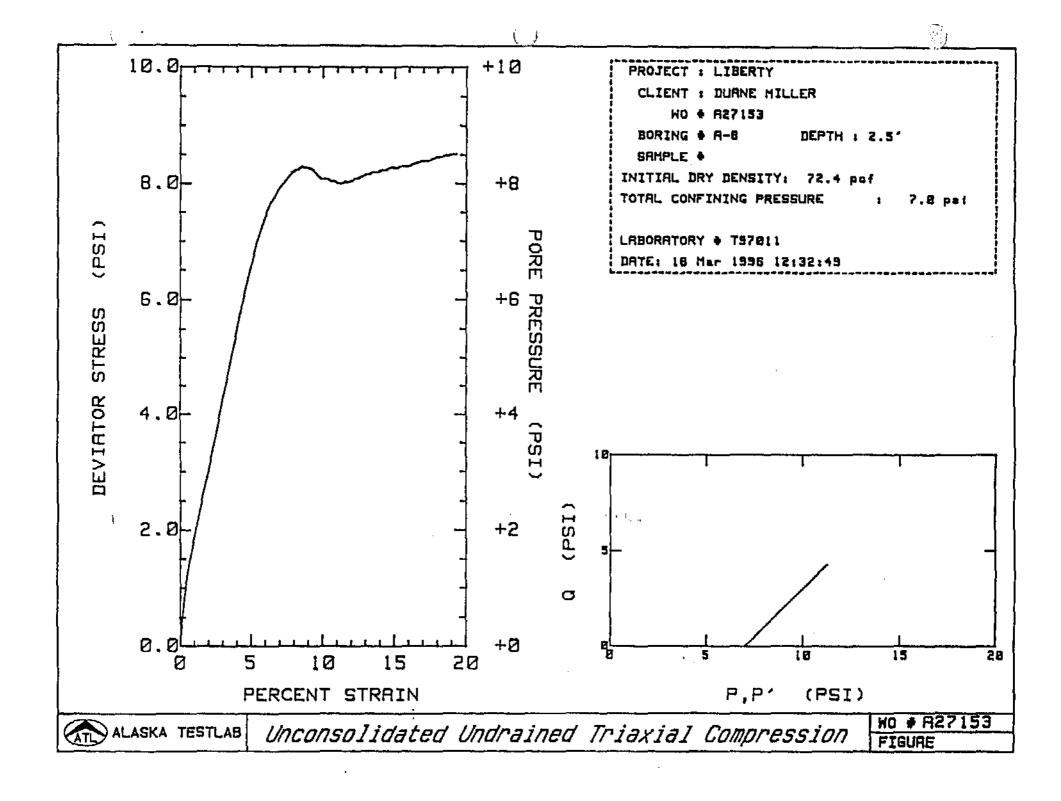
DATE : 18 Mar 1996 07:49:38



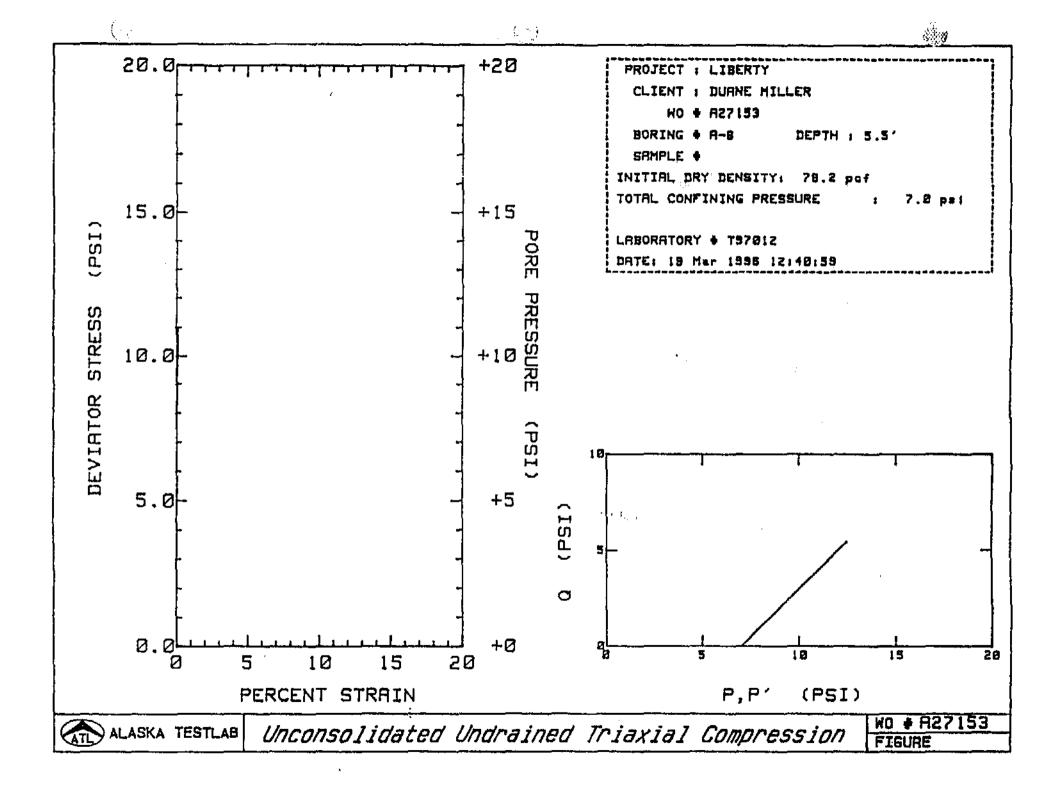
ALASKA TESTLAB

Mohr Diagram - UU Tests

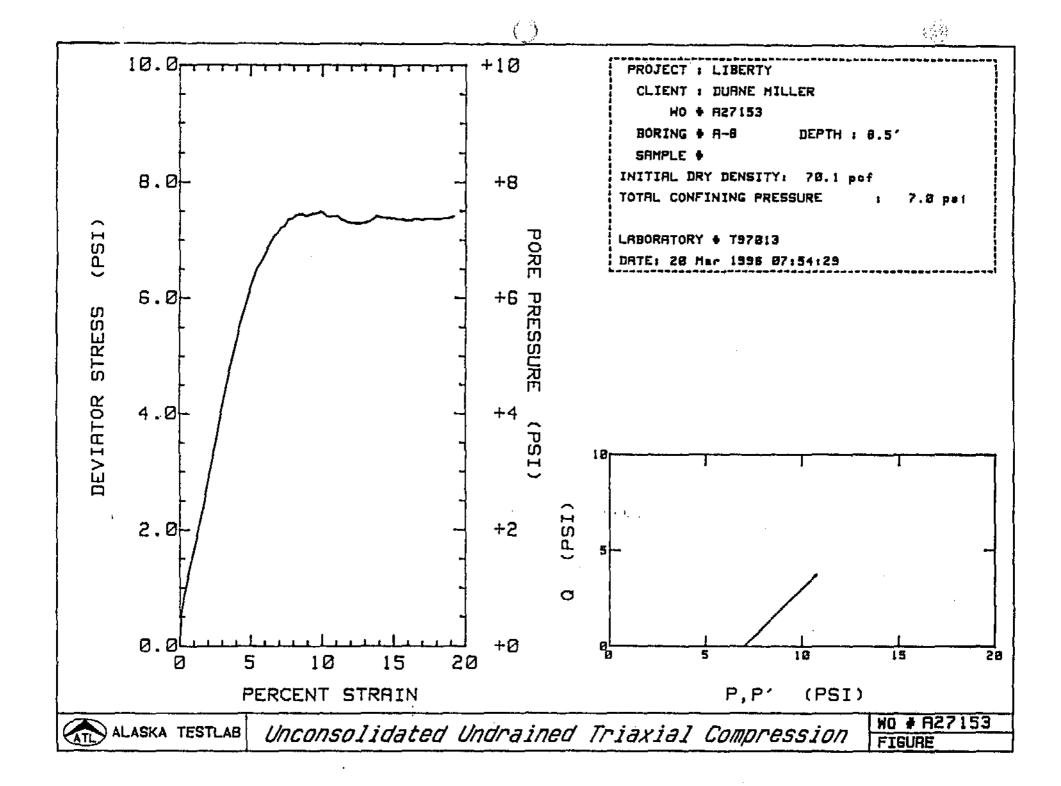
WO # A27153 FIGURE



LAB + T97011; 5 = 7.0 psi; 8 = 72.4 pcf PROJECT : LIBERTY CLIENT : DUANE MILLER W.O. # A27153 BORING # A-8 SAMPLE # DEPTH: 2.5' DATE: 18 Mar 1996 12:32:49 10 (PSI) STRESS SHEAR 10 20 NORMAL STRESS (PSI) WO # A27153 ATL ALASKA TESTLAB Mohr Diagram - UU Tests FIGURE



LAB # T97012; 5 = 7.0 psi; 8 = 78.2 pcf PROJECT : LIBERTY CLIENT : DUANE MILLER W.O. # #27153 BORING # A-8 SAMPLE # DEPTH: 5.5' DATE: 19 Mar 1996 12:40:59 10 (PSI) STRESS SHEAR 10 20 NORMAL STRESS (PSI) WO # A27153 ALASKA TESTLAB Mohr Diagram - UU Tests FIGURE

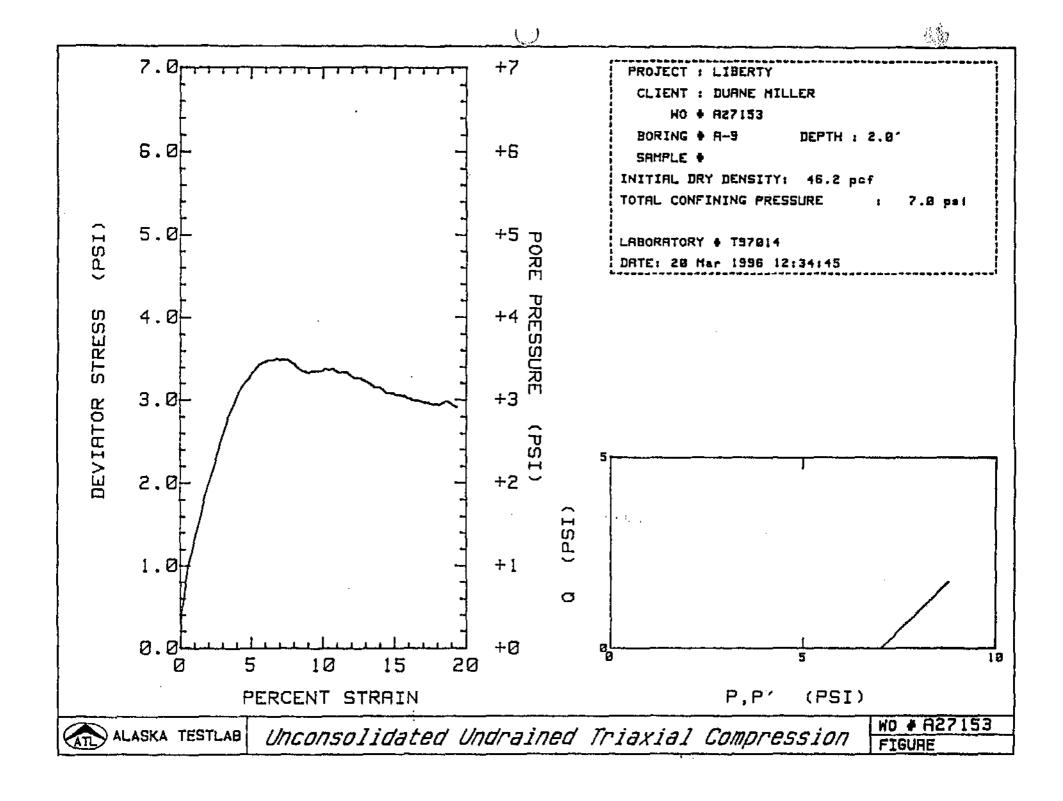


LAB # T97013; $O_{s} = 7.0 \text{ psi; } \delta_{s} = 70.1 \text{ pcf}$ PROJECT : LIBERTY CLIENT : DUANE MILLER W.O. # A27153 BORING # A-8 SAMPLE # DEPTH: 8.5' DATE: 20 Mar 1996 07:54:29 10 (PSI) STRESS SHERR 10 20 NORMAL STRESS (PSI)

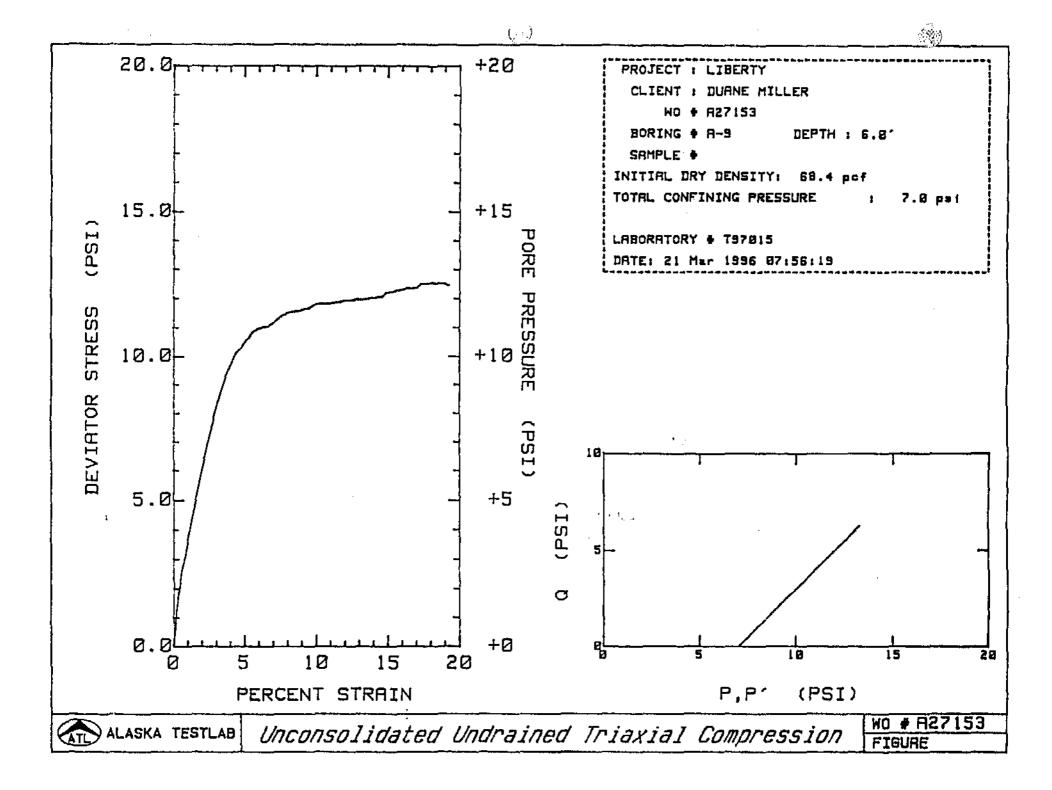
ALASKA TESTLAB

Mohr Diagram - UU Tests

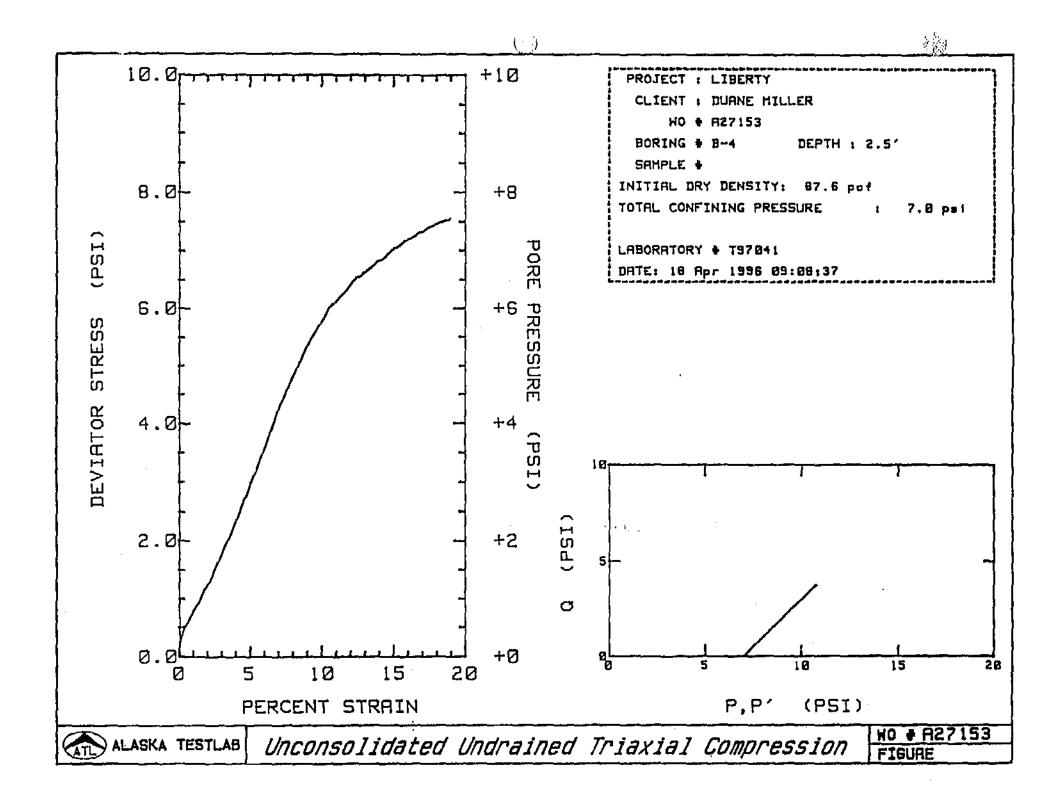
WO # A27153 FIGURE



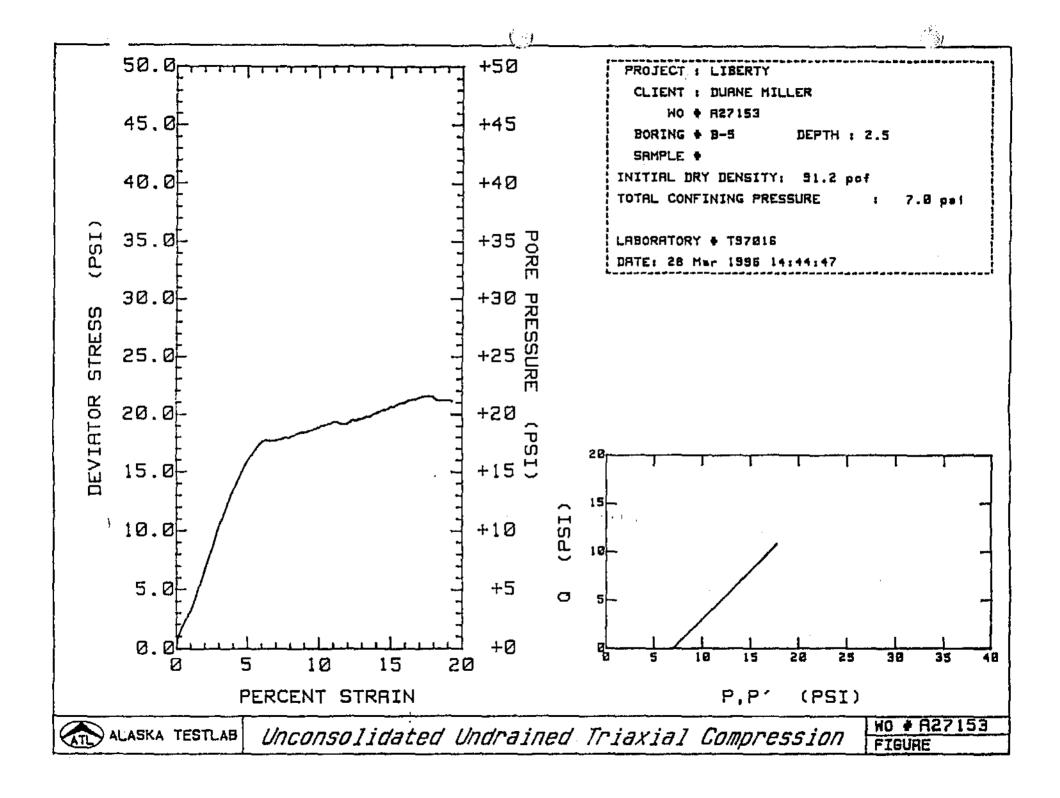
LAB # T97014; 0 = 7.0 psi; 8 = 46.2 pcf PROJECT : LIBERTY CLIENT : DURNE MILLER W.O. # A27153 BORING # A-9 SAMPLE # DEPTH: 2.0' DATE: 20 Mar 1996 12:34:45 10 (PSI) STRESS SHEAR 10 20 NORMAL STRESS (PSI) WO # A27153 Mohr Diagram - UU Tests ATL ALASKA TESTLAB FIGURE



LAB + T97015; G = 7.0 psi; 8 = 68.4 pcf PROJECT : LIBERTY CLIENT : DUANE MILLER W.O. # #27153 BORING # R-9 SRMPLE # DEPTH: 6.0' DRTE : 21 Mar 1996 07:56:19 10 (PSI) STRESS SHEAR 10 20 NORMAL STRESS (PSI) KO # A27153 Mohr Diagram - UU: Tests ATL ALASKA TESTLAB FIGURE



LAB # T97041; 5 = 7.0 psi; 8 = 87.6 pcf PROJECT : LIBERTY CLIENT : DUANE MILLER W.O. # R27153 BORING # B-4 SAMPLE # DEPTH: 2.5' DATE: 18 Apr 1996 05:08:37 10 (PSI) STRESS SHERR 10 20 NORMAL STRESS (PSI) WO # A27153 Mohr Diagram - UU Tests ALASKA TESTLAB FIGURE



LAB # T97016; 0 = 7.0 psi; 8 = 91.2 pcf

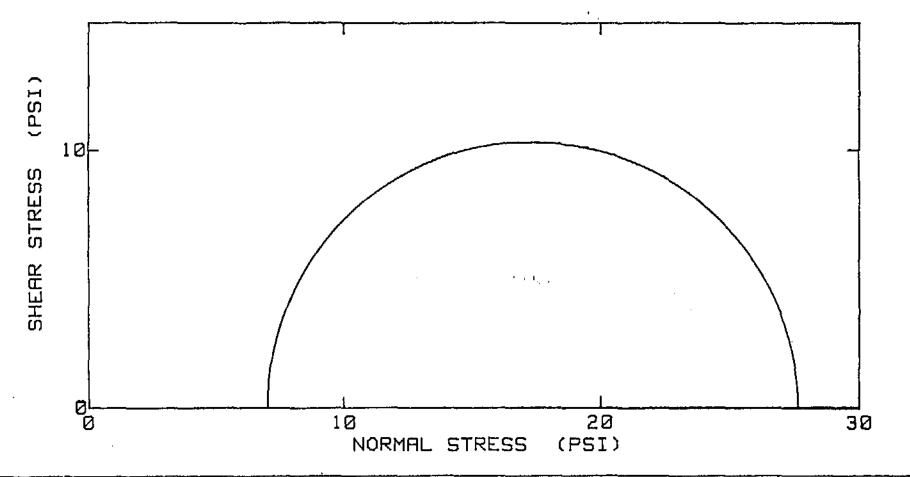
CLIENT : DURNE MILLER

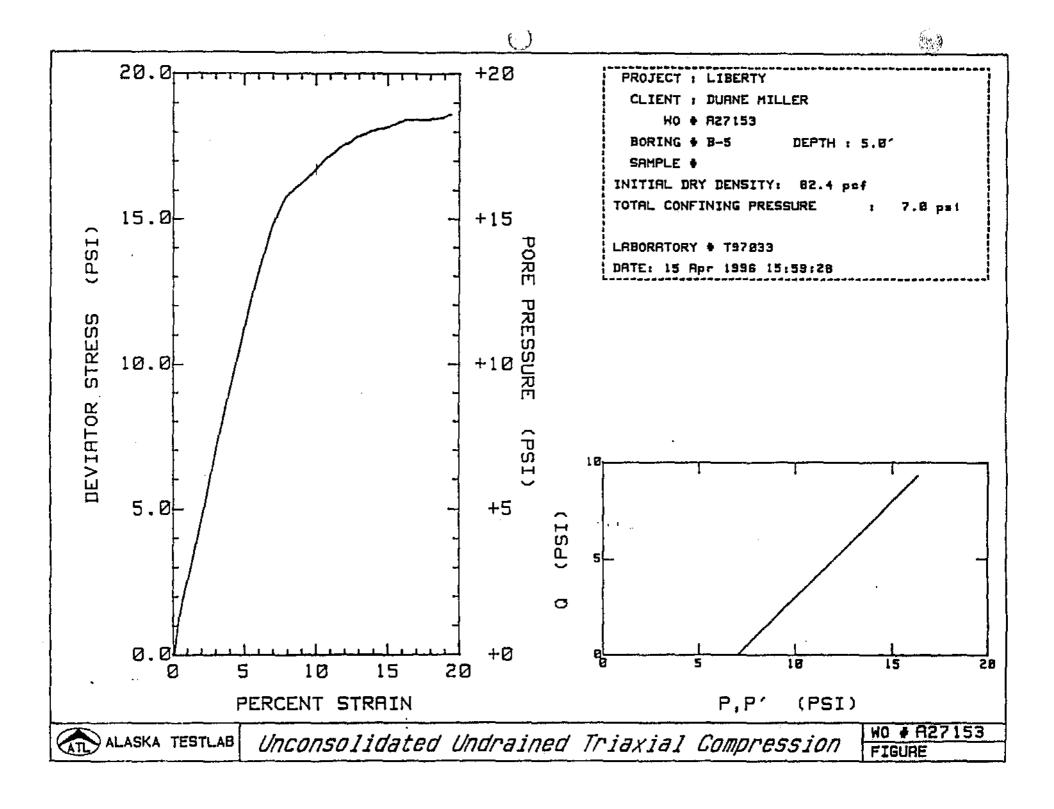
W.O. # R27153 BORING # B-5

SAMPLE #

DEPTH: 2.5

DRTE: 28 Mar 1995 14:44:47





LAB # T97042; 👣 - 7.0 pst; 🖔 = 91.9 pcf

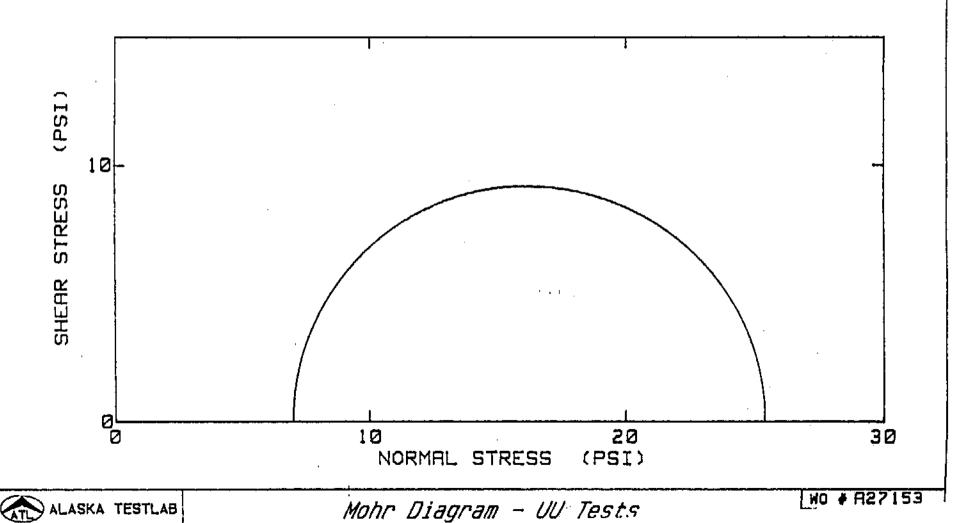
CLIENT : DURNE MILLER

W.O. # R27153 BORING # B-5

SAMPLE #

DEPTH: 7.3'

DATE : 18 Apr 1996 10:01:35



LAB # T97042; 0 = 7.0 psi; 0 = 91.9 pcf

CLIENT : DURNE MILLER

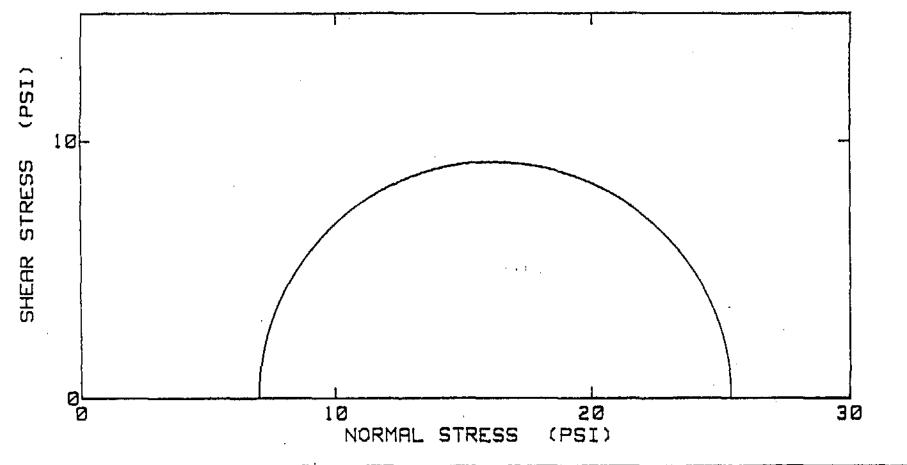
W.O. # A27153

BORING # B-6

SAMPLE #

DEPTH: 7.3'

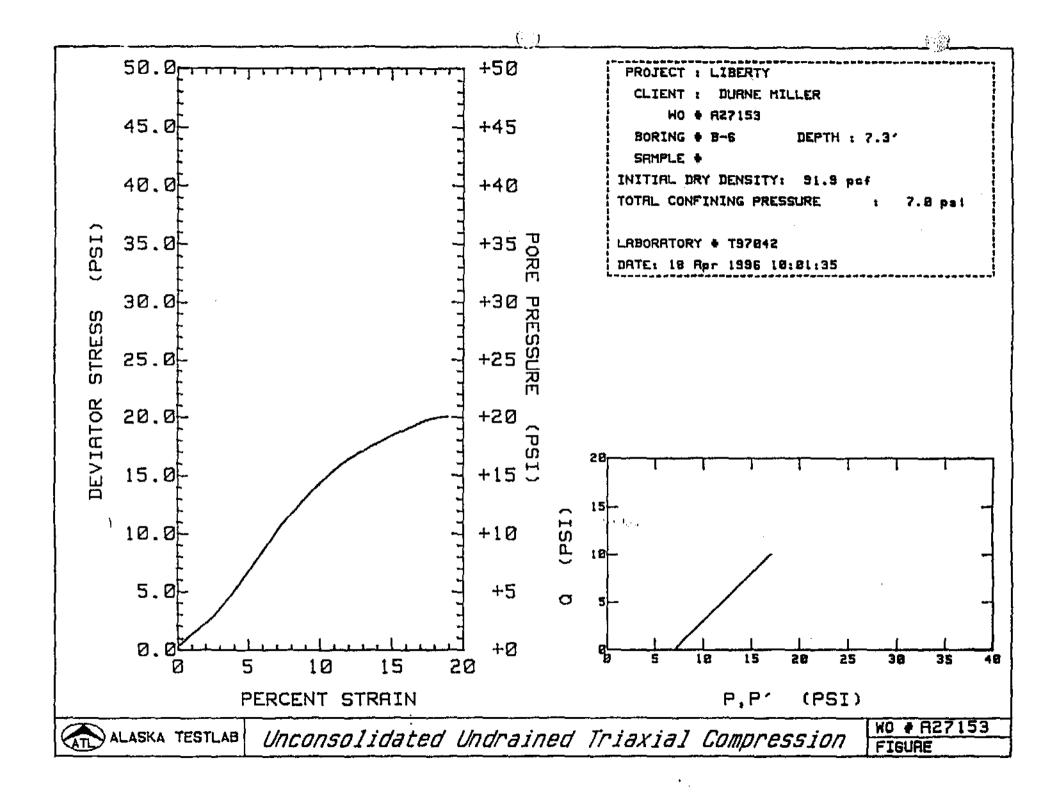
DATE : 18 Apr 1996 10:01:35



ALASKA TESTLAB

Mohr Diagram - UU Tests

WO # A27153



LAB # T97042; 5 = 7.0 psi; 8 = 91.9 pcf

FIGURE

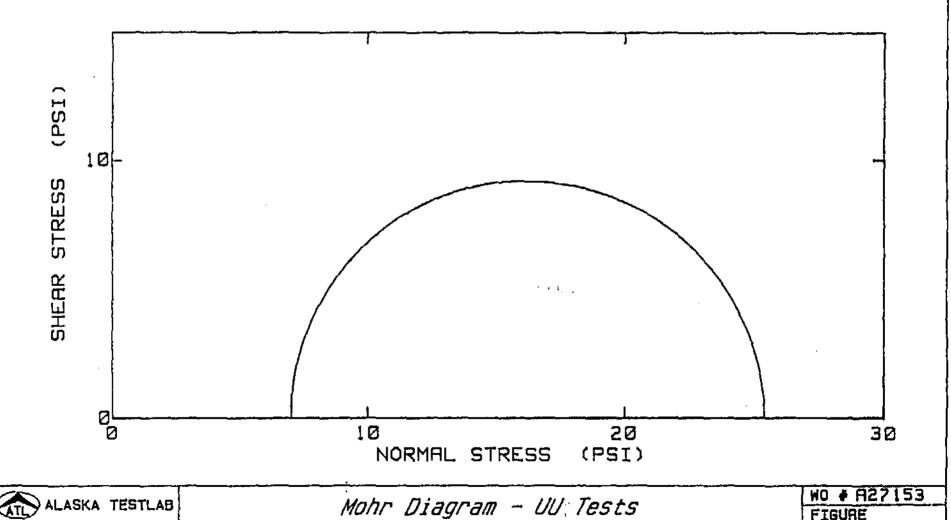
CLIENT : DUANE MILLER

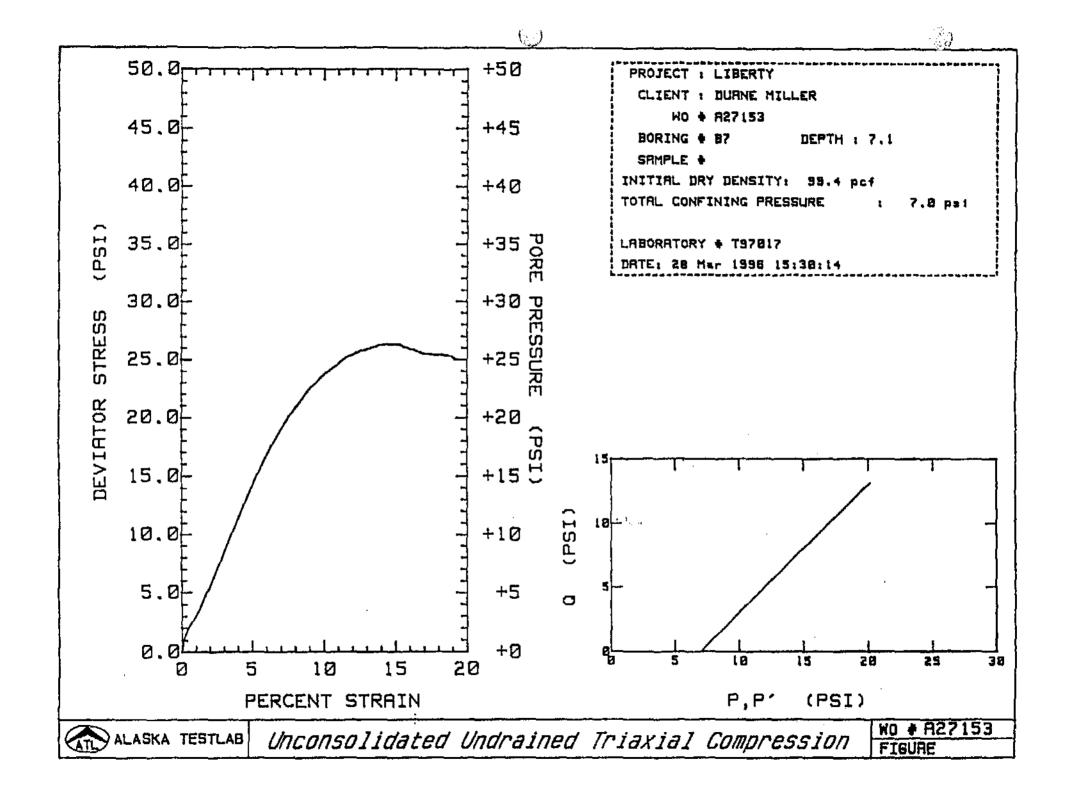
W.O. # R27153 BORING # B-6

SAMPLE #

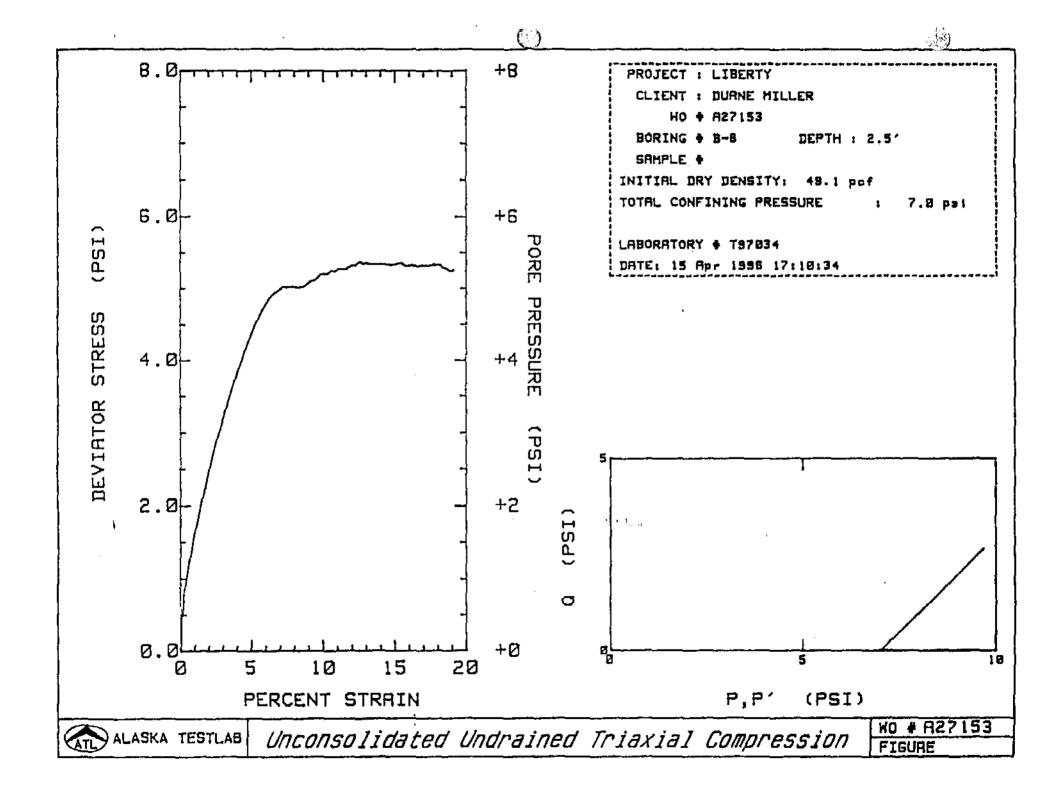
DEPTH: 7.3'

DATE: 18 Apr 1996 10:01:35





LAB # T97017; $Q_{3} = 7.0 \text{ psi; } X_{4} = 99.4 \text{ pcf}$ PROJECT : LIBERTY CLIENT : DUANE MILLER W.O. # F127153 BORING # B7 SAMPLE # DEPTH: 7.1 DATE: 28 Mar 1996 15:30:14 20 (PSI) STRESS 10-SHEAR 10 20 30 40 NORMAL STRESS (PSI) WO # A27153 Mohr Diagram - UU Tests ALASKA TESTLAB FIGURE



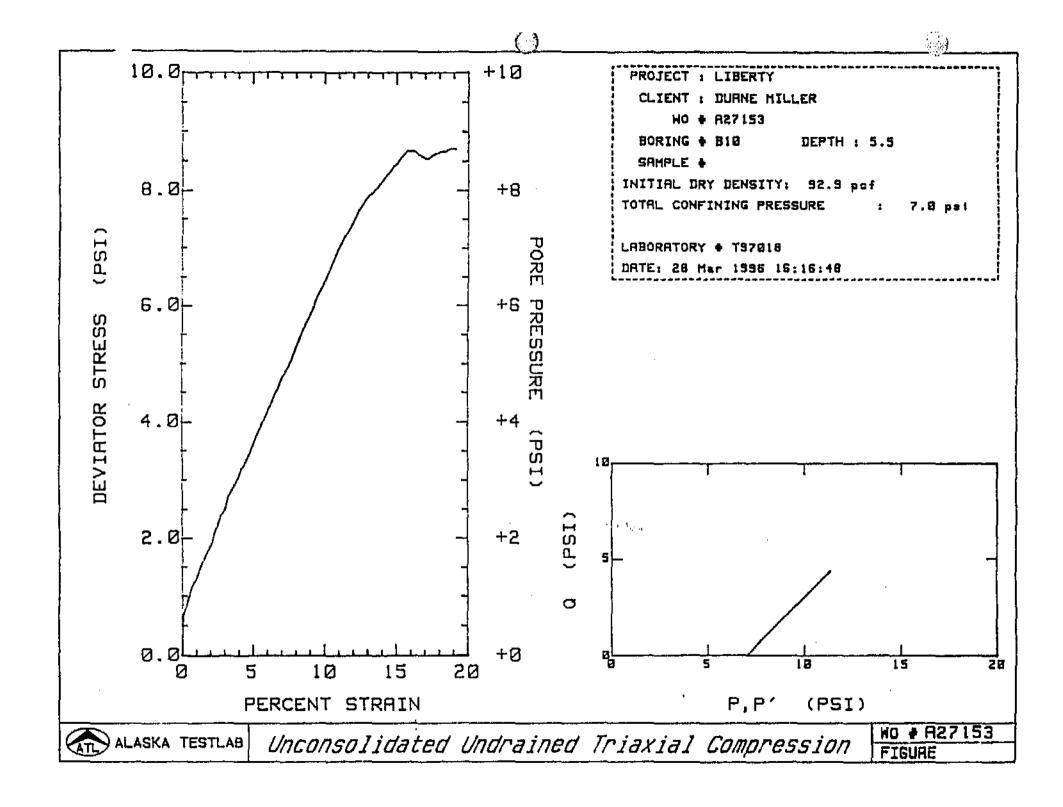


()LRB # T97034; $\delta_{3} = 7.0 \text{ psi; } \delta_{d} = 49.1 \text{ pcf}$ PROJECT : LIBERTY CLIENT : DUANE MILLER W.O. # #27153 BORING # B-B SAMPLE # DEPTH: 2.5' DATE: 15 Apr 1995 17:10:34 10 (ISd) SHERR STRESS 10 20 NORMAL STRESS (PSI)

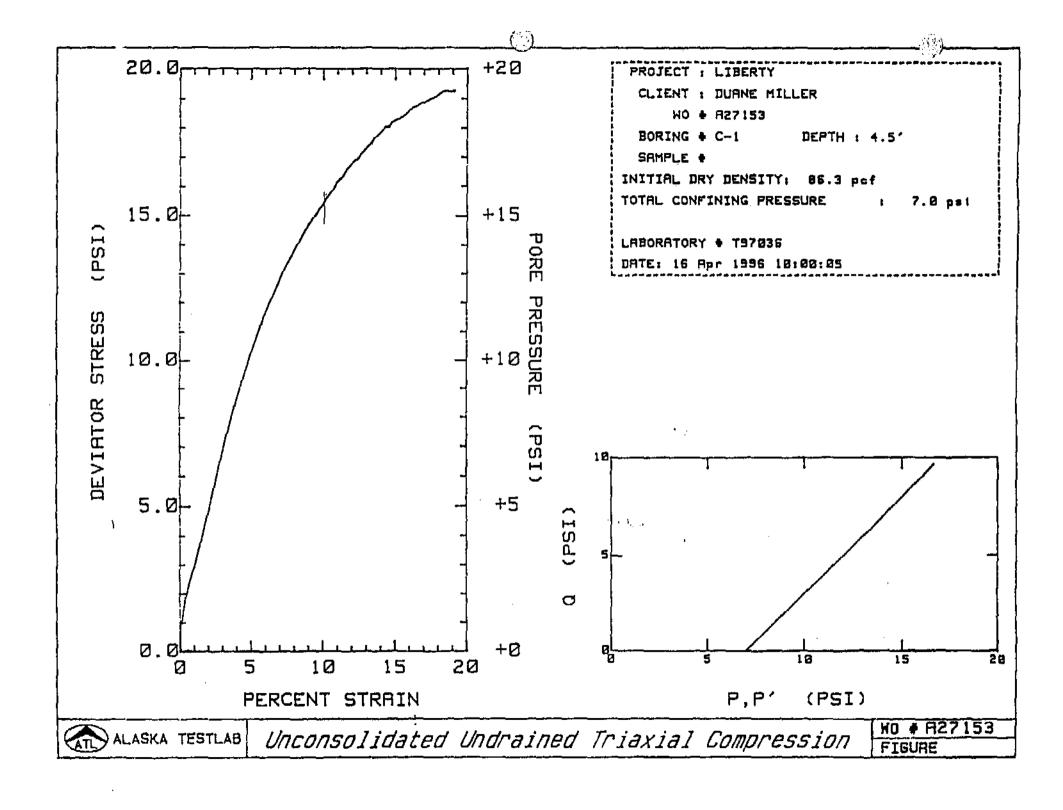
ALASKA TESTLAB

Mohr Diagram - UU Tests

WO # 827153 FIGURE



LAB + T97018; 5 = 7.0 psi; 8 = 92.9 pcf PROJECT : LIBERTY CLIENT : DURNE MILLER W.O. # FI27153 BORING # B10 SAMPLE # DEPTH: 5.5 DATE: 28 Mar 1996 16:16:48 10 (PSI) STRESS SHEAR 10 20 NORMAL STRESS (PSI) WO # A27153 ALASKA TESTLAB Mohr Diagram - UU Tests FIGURE

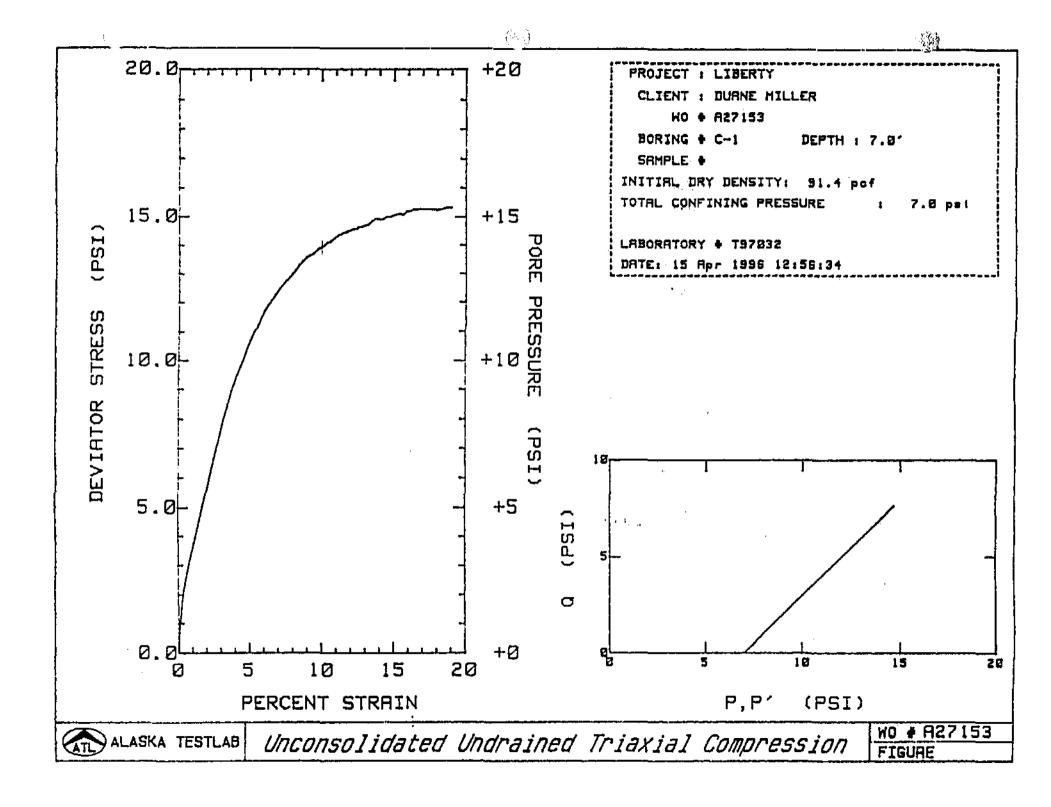


LAB # T97036; 5 = 7.0 psi; 8 = 86.3 pcf PROJECT : LIBERTY CLIENT : DURNE MILLER W.O. # A27153 BORING # C-1 SAMPLE # DEPTH: 4.5' DATE : 16 Apr 1996 10:00:05 (PSI) 10-STRESS SHEAR 10 20 30 NORMAL STRESS (PSI)

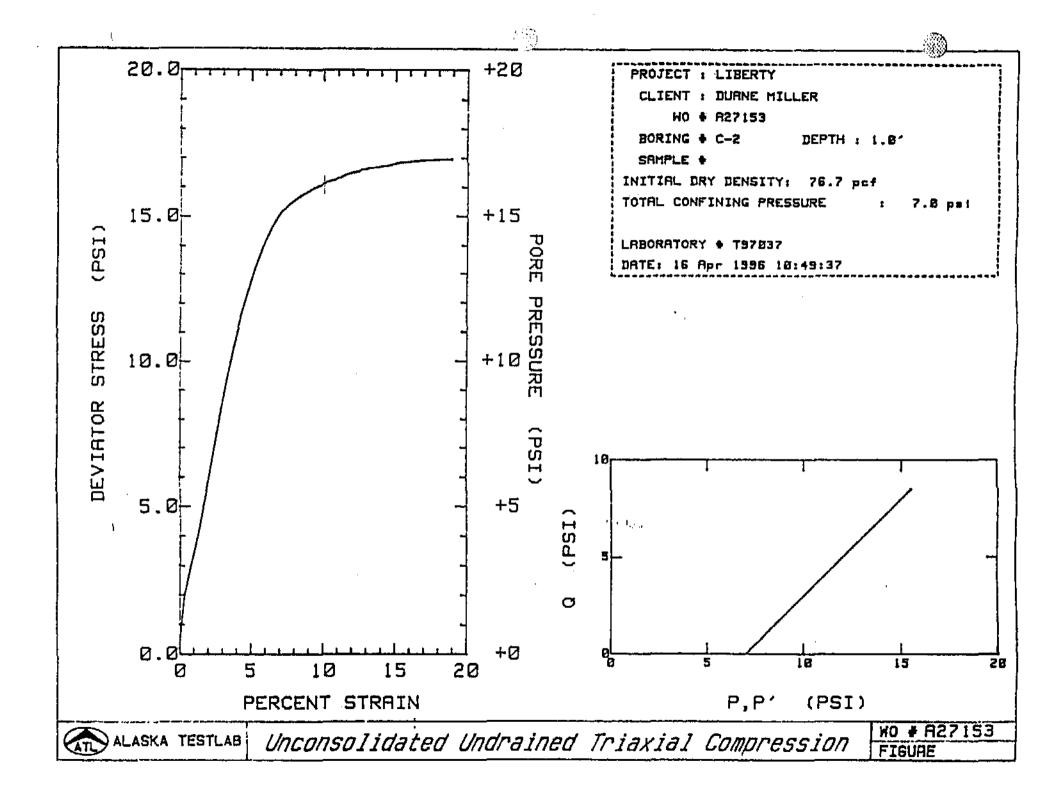
ALASKA TESTLAB

Mohr Diagram - UU: Tests

WO # A27153 FIGURE



LAB # T97032; 0 = 7.0 psi; 0 = 91.4 pcf PROJECT : LIBERTY CLIENT : DURNE MILLER W.O. # R27153 BORING # C-1 SAMPLE # DEPTH: 7.0' DATE : 15 Apr 1996 12:56:34 (PSI) 10-STRESS SHEAR 10 20 30 NORMAL STRESS (PSI) WO # F127153 ALASKA TESTLAB Mohr Diagram - UU Tests FIGURE



LRB + T97037; $\delta_{i} = 7.0 \text{ psi; } \delta_{i} = 76.7 \text{ pcf}$

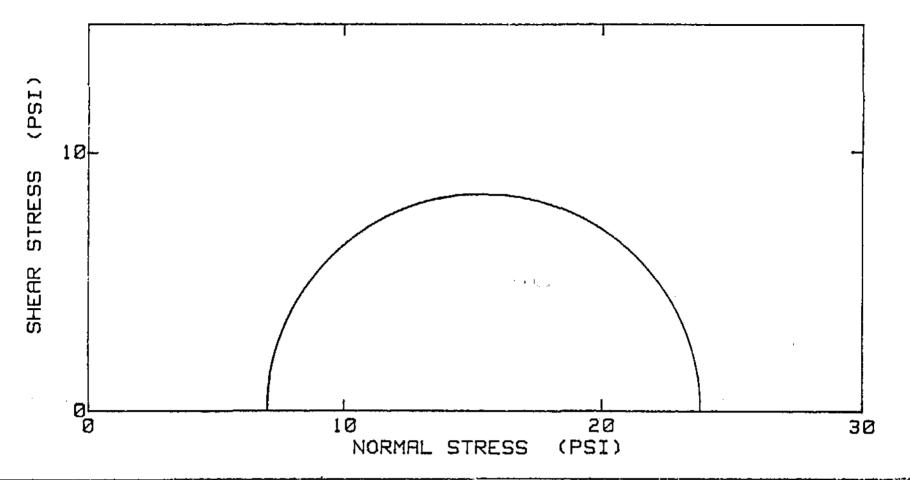
CLIENT : DURNE MILLER

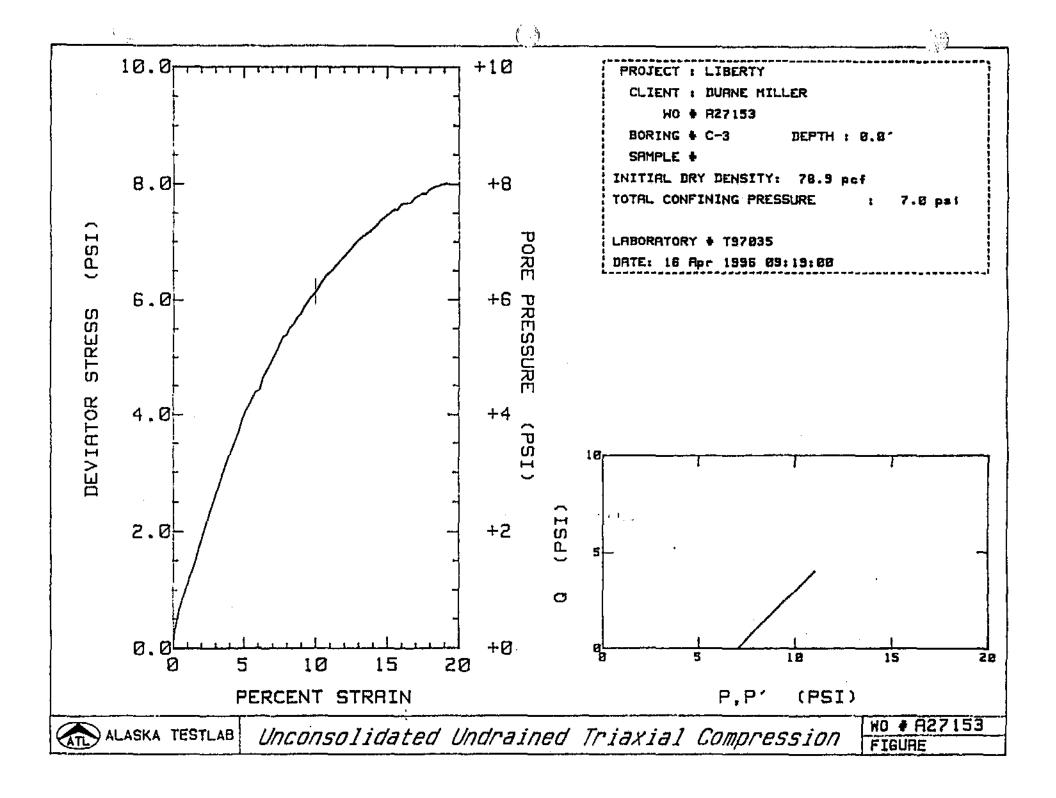
W.O. # A27153 BORING # C-2

SRMPLE #

DEPTH: 1.0'

DATE : 16 Apr 1996 10:49:37





LAB # T97035; Q = 7.0 psi; X = 78.9 pcf

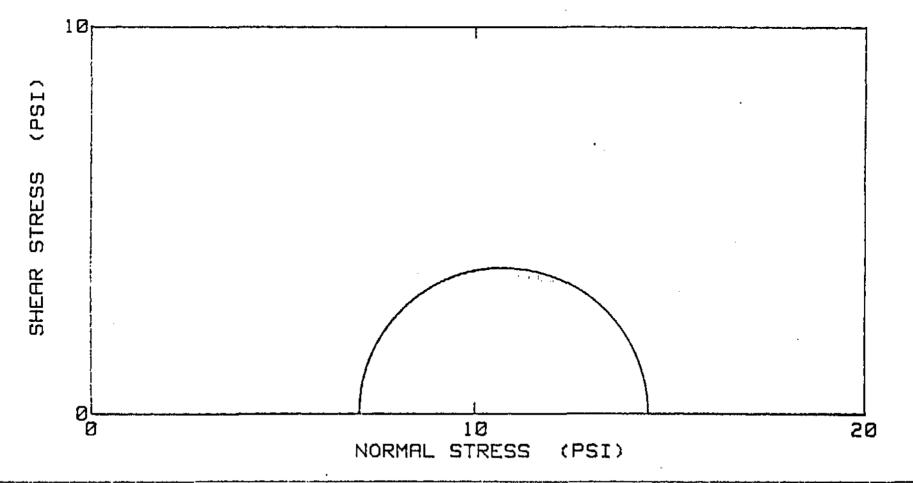
CLIENT : DURNE MILLER

W.O. # M27153 BORING # C-3

SAMPLE #

DEPTH: 0.01

DATE : 16 Apr 1996 09:19:00



ALASKA TESTLAB

Mohr Diagram - UU Tests

WO # A27153 FIGURE